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RICHELIEU RIVER BASIN EAST MONTPELIER

MONTPELIER NO. 4 VT. 00048

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

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DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASS, 02154_____

FEBRUARY 1979

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Richelieu River Basin East Montpelier, Vermont Winooski River

20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

The dam is a concrete gravity dam with an overall length of 152 ft. and a height of 21 ft. The dam is classified as small and has a low hazard potential in the ment of a dam failure. The dam is in poor condition. The lack of maintenance during the last 15-20 years may have rendered the dam economically unsalvageable. To avoid substantial costs to rehabilitate this dam, it is recommended that the dam dam be breached. A program of annual periodic technical inspections should be instituted.

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Chief. Design Branch Chief, F & M Branch Chief, Water Control Branch	Chairman, Dam Safety Revie	w Board		
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DEPARTMENT OF THE ARMY

NEW ENGLAND DIVISION, CORPS OF ENGINEERS 424 TRAPELO ROAD

WALTHAM, MASSACHUSETTS 02154

JUN 1 1 1775

Honorable Richard A. Shelling Governor of the State of Vernont State Capitol Hontpelier, Vernont 05602

Dear Sevarior Shelling:

I am forwarding to you a copy of the Montpelier No. 4 Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Water Desources, the cooperating agency for the State of Vermont. In addition, a copy of the report has also been furnished the owner, Green Mountain Power Corporation, Montpelier, Vermont 05602.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Water Resources for your cooperation in carrying out this program.

Sincerely yours,

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Grings, Greja of Tajiniera

Division Engineer

MONTPELIER NO. 4 DAM
VT. 00048

EAST MONTPELIER, VERMONT

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PHASE I INSPECTION REPORT

NATIONAL DAM INSPECTION PROGRAM

NATIONAL DAM INSPECTION PORGRAM

PHASE I INSPECTION REPORT

Identification No: VT 00048

Name of Dam: Montpelier No. 4 Town: East Montpelier

County and State: Washington County, Vermont

Stream: Winooski River Date of Inspection: November 21, 1978

BRIEF ASSESSMENT

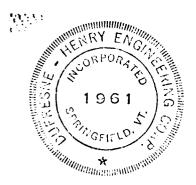
This dam is a concrete gravity dam with an overall length of 152 feet, a basal spillway width at the concrete/bedrock interface at the gate house abutment of 11 feet and a structural height of 21 feet. This dam was constructed by Cory, Devitt and Frost, Inc. for the former Montpelier and Barre Light and Power Company. At the right abutment there is a concrete structure housing the forebay and gates. This structure formerly diverted water to a penstock for use at a downstream power house. Green Mountain Power Corporation now owns the facility; however, its function as a low-head hydroelectric facility was discontinued around 1960.

The dam is classified as small and has a low hazard potential in the event of dam failure. The dam is designed to overtop continuously without structural damage. The dam is anchored directly into existing channel bedrock. The gate house formerly abutted the downstream portion of a bend in the river. This bend was overtopped by river flows, partially eroded away, and subsequently backfilled with soil fines to construct the present access road.

Based on size and hazard classification and in accordance with Corps Guidelines, the test flood is the 100-year flood. The test flood outflow of 17,000 CFS (85 CSM) overtops the dam by 9.1 feet. With water level at the top of the dam (elevation 616.2 feet MSL) the spillway will pass 8,700 CFS which is 50 percent of the test flood.

The dam is in poor condition. The lack of maintenance during the last 15-20 years may have rendered the dam economically unsalvageable. The downstream portion of the soils to the right of the gate house appear to be highly susceptible to erosion, and the upstream portion is of unknown composition. In addition, the concrete overflow weir may be unstable against overturning and horizontal sliding under high water conditions, particularly since silt behind the weir practically reaches to its crest. To avoid substantial costs to rehabilitate this dam, it is recommended that the dam be breached. Alternatively, the stability of the dam and the right abutment must be checked in detail. The following items require immediate attention:

- The waste gate and penstock gates should be opened permanently to maintain lower upstream water levels during normal flows. Prior to opening either gate, the Vermont Division of Water Resources should be contacted to specify necessary measures to control the release of silt build-up.
- 2. The forebay cover should be renovated to prevent unauthorized access and hazard to trespassers.
- The gate house should be disassembled and removed from the site.
- 4. The trees that are growing on the embankment to the right and upstream from the gate house should be cut annually.
- 5. The downstream slope of the embankment to the right of the gate house should be kept free of debris and trees so that any seepage through the embankment will be observable.
- 6. All deteriorated concrete at the dam and abutments must either be repaired or replaced if the dam is to remain in use.
- 7. A program of annual periodic technical inspections should be instituted.





This Phase I Inspection Report on Montpelier Number Four has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

MOSTPH W. FINEGAN, JR., MEMBER
Warer Control Branch
Engineering Division

Joseph Q. Mc Elroy

JOSEPH A. MCELROY, MEMBER Foundation & Materials Branch Engineering Division

CARNEY M. TERZIAN, CHAIRMAN

Chief, Structural Section

Design Branch

Engineering Division

APPROVAL RECOMMENDED:

COE B. FRYAR

Chief, Engineering Division

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test Flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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d. Reservoir Area

The reservoir area consists of approximately 4 acres when the water level is at spillway crest elevation, 610.6 feet MSL. This area was 7 acres in 1910 and has decreased due to heavy siltation.

e. Downstream Channel

The downstream channel is in good condition. There is very little debris or overhanging vegetation in or near the channel. There are several homes on the left side of the channel; however these homes are located above flood hazard levels. U.S. Route 2 crosses the Winooski River approximately 1 mile east of the dam, prior to the confluence of the Winooski River and Stevens Brook.

3.2 Evaluation

Montpelier No. 4 Dam has gradually deteriorated to a point such that renovation is not considered reasonable. The concrete has so severely spalled and cracked that actual replacement of the entire gate house and foundation would be necessary. Years without maintenance have rendered both the gate house and forebay cover unsalvageable.

The sluice gates and conduit equipment at the dam itself are no longer operable. The two sluice gates are not only inoperable but also beyond the point of repair. The penstock itself has been severed at the right abutment and has been completely removed downstream of the dam.

Based on the condition of the dam and the right abutment, it appears that this structure may be on the verge of incipient failure. However, the potential for destruction is extremely low due to the small volume of water behind the dam relative to the size of the downstream channel.

Based on the visual observations and the verbal information provided, it appears that the right abutment of this dam is uncompacted silt and trash fill that was not built as a dam but only as a road to access the gate house. Furthermore, no information is available on the character of the natural materials upstream from this access road. Therefore, it is not known whether these materials form an adequate dam. If the river overtops the right abutment again, it should be assumed that a washout will occur.

The left abutment of the dam appears to be composed of bedrock which appears to be competent.

the downstream side of the bend and worked upstream toward the headwater of the ice jam. Apparently a channel was not formed through the entire abutment. It is not known whether the upstream portion of the bend is controlled by a shallow ledge, or whether the water did not flow over the top long enough to cut all the way through.

At the time of inspection an embankment about 100 feet long at elevation 618.2 feet MSL existed between the gate house and the natural hill further to the right. The exposed portion of the downstream face of this embankment is composed of silt, fine sand and trash. The zone downstream from this embankment has been used for dumping trash. The surfaces of the embankment are generally grassed or forested. The shortest distance from the downstream crestline of this embankment to the upstream crestline (i.e. the top of the river bank on the upstream side of the dam) was about 200 feet at the time of inspection.

The material downstream of the left abutment also has been eroded during a previous overtopping. However, since this wall was anchored directly to bedrock, the eroded material was not required for support of the dam or abutment.

c. Appurtenant Structures

The discharge channel immediately downstream of the dam has exposed bedrock at both left and right banks. The channel has a cobble bottom and is clear and unobstructed.

The dam, constructed as a "run-of-the-river dam" has no intake channel. However, the quiescent conditions of the impoundment have allowed significant siltation to occur upstream of the dam. This siltation has severely limited both the maximum usable volume and surface area of the impoundment.

The intake channel into the forebay has significant silt deposits upstream from the bar racks. This silt has deposited upstream of the waste gate to a level near the top of the gate. The waste gate seemed to be rearly watertight. However, flow channels have developed in the waste gate discharge passageway as recorded in Photo 8 in Appendix C.

There is a flow of 10-20 GPM passing through the penstock sluice gate. This flow is recorded in Photo 7, Appendix C.

The forebay was covered, which prevented visual inspection.

The wood frame control building has deteriorated significantly. Its concrete footing serving as part of the dam is spalled and cracked beyond economical repair.

SECTION 3 - VISUAL INSPECTION

3.1 Findings

a. General

In general, the Montpelier No. 4 Dam is in very poor condition. The concrete has spalled severely both on the dam and abutment structures. In 1949, Stephen H. Haybrook, hydraulic engineer, noted that "surface disintegration has reduced its section and is further reducing the structure to incipient failure." The dam has weathered an additional 30 years with no apparent maintenance since that evaluation.

b. Dam

The dam consists of a concrete overflow spillway. On the day of inspection the water surface was slightly above the spillway crest (elevation 610.2 feet MSL). The downstream face of the dam has weathered significantly (see Photo 6, Appendix C). The concrete overflow spillway and gate house structure have been anchored directly to the bedrock existing throughout the entire length of the flow channel. Inspection for possible seeps at the base of the dam was not possible due to the continuous overflow. However, there was no visible seepage at the bedrock-concrete interface at the left or right abutment walls.

A wet area was observed to the extreme right of the dam. A stream carrying less than 3 GPM collects flow and rejoins the Winooski River channel downstream of the dam. It is not known whether this water is seepage through the embankment from the river, or runoff and ground water from the adjacent natural land. The water in this stream is discolored with a rusty stain on the downstream end, and is relatively stagnant on the upstream end.

There has been extensive fill (sandy silt with wood trash) placed upstream of the wet area to the right of the dam. This fill seems to have been placed to repair the access road which was eroded during a previous overtopping.

Mr. Raymond C. DeForge, Vice President of Operations and Engineering, Green Mountain Power Corporation, has indicated that the natural materials in the abutments have been overtopped in the past due to an ice jam upstream from the dam.

As a result of that overtopping the soil that comprised the right abutment, which is the mass that forms the bend in the river at this location, was eroded away. Erosion started on

2.3 Operation Data

The dam was attended during the period of time it was in operation as a low-head hydro facility (1908-1960). During the last 20 years there has not been any program of operation in effect at Montpelier No. 4 Dam.

2.4 Evaluation of Data

a. Availability

The original power license application plans for Montpelier No. 4 Dam are stored in the archives at the Montpelier office of the Green Mountain Power Corporation. There are no design calculations relating to structural or hydraulic aspects of the dam itself.

b. Adequacy

Based on the visual inspection and the available engineering data (Figures 1 and 2, Appendix B), the information is sufficient for the Phase I inspection.

c. Validity

The available engineering data are considered valid on the basis of the visual inspection.

SECTION 2 - ENGINEERING DATA

2.1 Design Data

There are no design drawings or calculations available other than the "as built" drawings included in Appendix B, Figures 1 and 2. These drawings were part of the Green Mountain Power Corporation license application dated June 30, 1966.

A letter report, prepared in 1949 by Stephen H. Haybrook, contains a complete evaluation of the dam at that time. This report made reference to a report by Barker and Wheeler prepared in 1926. This report was not available for evaluation.

2.2 Construction Data

The dam was constructed in 1908 by Cory, Devitt and Frost, Inc. According to Green Mountain Power Corporation there has been little or no maintenance work on either the dam or related structures since construction.

There is no evidence of the dam being raised from its initial constructed elevation of approximately 610.2 feet MSL.

The concrete dam has been anchored into the existing bedrock as shown in Appendix B, Figure 2. In cross section, the concrete dam has faces symmetrical about the centerline and formed at an average slope of 4H:21V. There are flashboard support holes set into the crest of the dam at 15-foot intervals. The flashboards and supports are no longer in place.

The gate house footing and concrete penstock intake structure rest directly on the bedrock. A concrete training wall extends upstream about 75 feet from the gate house. This wall is shown in Appendix B, Figure 2, as not extending into bedrock.

The covering of the forebay is of wood construction with a tarpaper covering. The gatehouse is a single story wood frame building located directly over the air and gate shafts.

The 7-foot diameter steel penstock passing under the right end of the dam forms the forebay outlet and is hydraulically controlled by a sluice gate located at the penstock intake.

Also passing through the right end of the dam is a waste gate discharge passageway. This outlet is controlled at the upstream end by a wooden sluice gate. This structure provides a method of draining the impoundment for maintenance procedures.

(5) Side Slope

Upstream - approximately 4H:21V.

Downstream - approximately 4H:21V.

(6) Zoning

Not applicable.

(7) Impervious Core

Not applicable.

(8) Cutoff

Concrete to bedrock of stream channel.

(9) Grout Curtain

None known.

(10) Other

No other aspects known.

h. Diversion and Regulating Tunnel

Not applicable.

i. Spillway

The spillway consists of a modified ogee spillway 152 feet in length. The crest of the weir is about 3 feet wide and its upstream and downstream faces are sloped at about 4H:21V. The spillway is anchored directly into bedrock. The maximum height of the structure is about 21 feet.

j. Regulating Structures

The water level can be affected by opening or closing either the waste gate or penstock intake gate. Otherwise the impoundment level remains at or above the spillway crest elevation of 610.2 feet above MSL. Both gates are controlled by wooden sluice gates, now inoperable. The invert elevation of the 7' x 10' waste gate passageway is 592 feet MSL. The 7-foot diameter penstock also has an invert elevation of 592.0 feet MSL.

			Elevation (feet above MSL)
	Spil:	lway Crest	610.2
	Norma	al Pool	610.4
	Upst	ream Invert of Waste Gate Passageway	592
	Stre	ambed at Centerline of Dam	592
d.	Rese	rvoir Dam	Feet
	Leng	th of Test Flood Pool	1000
	Leng	th of Normal Pool	1000
e.	Stor	age Data	Acre-Feet
	Test	Flood (100-year)	51.6
	Top	of Dam (at left abutment)	39.2
	Norm	al Pool	16
f.	Rese	rvoir Surface Area	Acres
	Test	Flood (100-year)	4
	Тор	of Dam (at left abutment)	4
	Norm	al Pool	4
g.	Dam		
	(1)	<u>Type</u>	
		The dam is a concrete gravity dam.	
	(2)	Length	
		The overall length is about 227 feet; is 152 feet.	the weir length
	(3)	<u>Height</u>	
		The maximum height is 26.2 feet.	
	(4)	Top Width	
		A level section through the top of the about 3 feet.	rounded weir is

waste gate passageway is a 10' x 7' outlet controlled by a sluice gate which is presently inoperable. The intake into the forebay is controlled by another sluice gate. Invert elevations for both conduits are at approximately 592.25 feet MSL. Silt has essentially clogged the upstream side of these gates.

(2) Maximum Known Flood at Dam Site

There are no records available regarding major flooding at the dam site. Green Mountain Power Corporation personnel indicate that ice jams at the dam cause a much higher water level than a 100-year frequency flood.

(3) Spillway Capacity

The capacity of the spillway at elevation 618.2 feet MSL is about 13,400 CFS.

- (4) <u>Ungated Spillway Capacity During Test Flood</u>

 The ungated spillway capacity at the 100-year test flood elevation 619.3 feet MSL is approximately 16,000 CFS.
- (5) <u>Gated Spillway Capacity at Normal Pool Elevation</u>
 Not applicable.
- (6) Gated Spillway Capacity at Test Flood Elevation
 Not applicable.
- (7) Total Spillway Capacity at Test Flood Elevation
 16,000 CFS at elevation 619.3 feet MSL.
- (8) Total Project Discharge at Test Flood Elevation
 17,000 CFS at 619.3 elevation.

с.	Elevation Data	Elevation (feet above MSL)
	Top of Dam (Maximum) (at gatehouse and access road)	618.2
	Top of Dam (Minimum) (at left abutment)	616.2
	Top of Flashboards (no longer used)	612.7
	Test Flood (100-year)	619.3

g. Purpose of Dam

The dam was designed solely to provide an intake basin from which water could be withdrawn and transported to a power generation station located downstream. The impounded water entered the forebay through a bar rack structure and passed to a 7-foot diameter penstock. The penstock carried water approximately 0.53 miles westerly along the river to the Number 4 power station.

h. Design and Construction History

The dam was constructed in 1908 by Cory, Devitt and Frost, Inc. Design records are not available. There are two plans prepared for Federal Power Commission licensing available which seem to be in agreement with observations made during the visual inspection.

i. Normal Operational Procedures

The dam has not been operated during the past 15 years.

1.3 Pertinent Data

a. Drainage Area

The drainage area at the Montpelicr No. 4 Dam is 201 square miles. The flow at this site is regulated by Peacham Pond and Mollys Falls Reservoir (combined usable capacity 11,295 acre-feet). The Winooski River is located in the central portion of the physiographic region of Vermont known as the Vermont Piedmont. The river flows westerly into Lake Champlain and has an average slope upstream of the site of 13 feet per mile. The soils within the watershed are predominantly well drained loamy silty soils on gently sloping to steep hills.

The river channel in the vicinity of the dam is principally a calcareous sandstone known as the Waites Formation. Exposures of a nearly-vertically foliated graphitic phyllite and pegmatite intrusions are both present in the vicinity. The upstream side of the bend in the river that forms the right abutment of the dam is composed of glacial till. The thickness of this till is not known.

b. Discharge at Dam Site

(1) Outlet Works

The spillway is designed for continuous overtopping and thus provides the major outlet at Montpelier No. 4. In addition, there have been two other structures designed into the dam to function as outlets. The waste gate and

b. Description of Dam and Appurtenances

The Montpelier No. 4 Dam is a concrete gravity dam with an overall length of 152 feet, a basal spillway width of 11 feet and a structural height of 21 feet.

The modified ogee spillway is anchored directly into the channel bedrock. The left abutment is bedrock and the right abutment consists of soil.

There are two low level outlets built into the dam, both controlled by wooden sluice gates. The waste gate passageway is a 10' x 7' outlet at invert elevation 592.25 feet MSL. The other outlet conduit was initially designed as the 7-foot diameter penstock intake at invert elevation 592.00 feet MSL. The penstock has been severed at the downstream gate house abutment.

The downstream channel is clear of debris and overhanging forestation and slopes at approximately 3 feet per 1000 feet.

c. Size Classification

The Montpelier No. 4 Dam has a size classification of small. The dam impounds about 700,000 cubic feet (16 acre-feet) at a normal water surface of 610.4 feet MSL. The storage with a water surface at the top of the dam (elevation 618.25) is about 48 acre-feet. A dam with a maximum storage volume of less than 1000 acre-feet or a height of greater than 25 feet but less than 40 feet is classified as small. In this case both criteria apply.

d. Hazard Classification

The potential for hazard in the event of failure of this dam is low. Homes in the area affected are above the flood wave.

e. Ownership

The dam is owned by the Green Mountain Power Corporation, Montpelier, Vermont.

f. Operator

Montpelier No. 4 Dam has no one individual responsible for the day-to-day operation of the facility. Questions regarding operational procedures should be addressed to Green Mountain Power Corporation, telephone 802-223-5235.

NATIONAL DAM INSPECTION PROGRAM
PHASE I INSPECTION REPORT
NAME OF DAM: MONTPELIER NO. 4

SECTION 1 - PROJECT INFORMATION

1.1 General

a. Authority

Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a National Program of Dam Inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Dufresne-Henry Engineering Corporation has been retained by the New England Division to inspect and report on selected dams in the State of Vermont. Authorization and notice to proceed were issued to Dufresne-Henry Engineering Corporation under a letter of November 20, 1978 from Max B. Scheider, Colonel, Corps of Engineers. Contract No. DACW33-79-C-0010 has been assigned by the Corps of Engineers for this work.

b. Purpose

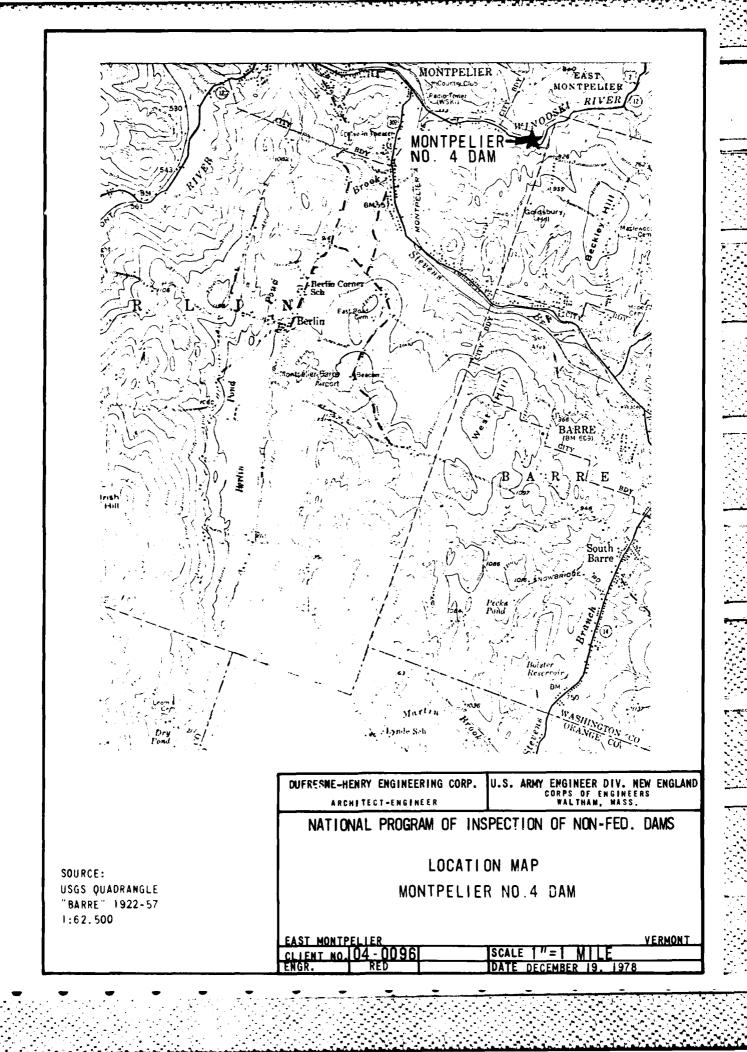
- (1) Perform technical inspection and evaluation of nonfederal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by nonfederal interests.
- (2) Encourage and prepare the states to initiate quickly effective dam safety programs for nonfederal dams.
- (3) To update, verify and complete the National Inventory of Dams.

1.2 Description of Project

a. Location

Montpelier No. 4 Dam is located partially in the Towns of Berlin and East Montpelier, Washington County, Vermont.

The site is located on the Winooski River which empties into Lake Champlain and is approximately 7,400 feet upstream from the confluence of the Winooski River and Stevens Brook.





OVERVIEW OF

MONTPELIER NO. 4 DAM

EAST MONTPELIER, VERMONT

Section			Page
7. ASSES	SMENT, RECOMMENDATIONS	AND REMEDIAL MEASURES	
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	a. Conditionb. Adequacy of Informc. Urgencyd. Need for Additional		7-1 7-1 7-1 7-1
7.2	Recommendations		7-1
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SECTION 4 - OPERATIONAL PROCEDURES

4.1 Procedures

The dam has not been operated for the past 15 to 20 years.

4.2 Maintenance of the Dam

During the time of operation, routine maintenance was necessary to remain economical as a low-head hydro intake facility. However, during the past 15-20 years the dam has not been used and therefore not maintained.

4.3 Maintenance of the Operating Facilities

There is no maintenance provided.

4.4 Description of any Warning System in Effect

There is no warning system in effect.

4.5 Evaluation

There have been little or no operation or maintenance procedures during the past 15-20 years.

SECTION 5 - HYDRAULIC/HYDROLOGIC

5.1 Evaluation of Features

a. General

The Montpelier No. 4 Dam is a concrete gravity dam of the run of the river type. It is basically a low surcharge storage-high spillage dam designed for continual overtopping.

b. Design Data

No design data for the structures at Montpelier No. 4 are available.

c. Experience Data

There is no written record of overtopping at Montpelier No. 4. It experienced the November 1927 flood without significant damage even though Montpelier No. 5 upstream of this site was severely damaged during the flood. There is some possibility that Mollys Falls Reservoir moderated the peak flow, but hydrologic analyses indicate that the dam must have been overtopped in 1927.

Green Mountain Power Company representatives indicate that an ice jam occurred in the mid-1930s which caused the overtopping that eroded substantial portions of the embankment on the right abutment. There are also positive signs of overtopping on the left abutment. This may have occurred either in 1927 or probably 1936.

d. Visual Observations

The continual overtopping of the Montpelier No. 4 Dam may continue for many years without failure. However, the flow channels through the concrete dam indicate significant internal deterioration. Spalling on the downstream side of the spillway has significantly reduced the effective dam section. The deteriorated condition of the concrete coupled with inadequate earthen dike on the right upstream river bank render the future of Montpelier No. 4 Dam precarious.

e. Test Flood Analysis

The test flood was selected as the 100-year flood using data provided by a Flood Insurance Study prepared by the Federal Insurance Administration by the New York District, Corps of Engineers in May 1975. The 100-year flood at the dam site is approximately 17,000 CFS (85 CSM) and under this condition the dam would be surcharged by 9.1 feet of water. This flow was not adjusted for surcharge storage as this is a run-of-the-river impoundment with low storage capacity.

f. Dam Failure Analysis

Under normal conditions, if the Montpelier No. 4 Dam were to fail due to the excessive deterioration of the concrete, a section approximately 20 feet wide near the spillway would fail first. This failure would release approximately 3500 CFS of water. Under the test flood conditions, if the dam were to fail approximately 5800 CFS additional flow would be released. Under these same conditions, if the dam held and the earthen dike on the right bank were to fail, 2600 CFS of water would escape. However, regardless of failure of either structure during the test flood, the City of Montpelier for the 100-year flood would already be under water. Therefore, the failure of either the dam or the erosion of the dike would have minimal additional impact on the City of Montpelier. Failure of either a 20-foot section of the concrete spillway or 60 feet of the earth embankment will cause a 1-foot or less rise in the 100year flood level along the Winooski River upstream of the North Branch-Winooski River.

Sixty (60) feet of the concrete spillway failing with normal water levels would release a peak rate of 10,540 CFS and releasing 16 acre-feet of impounded water. This would create a wave of water about 9 feet high based on section backwater at the site. At this depth the Winooski River impounds about 20 acre-feet over an 845-foot reach. This impoundment capacity along the reach of the river would rapidly reduce the flood wave so that it would not overtop the river banks which are generally 15 feet or more above the stream bed.

For the water to be at the top of dam, elevation 618.2, the Winooski River would have to be discharging approximately 13,400 CFS. A 60-foot breach forming in the dam with water at this stage would release an additional 8,300 CFS for a total flow in the river of 21,700 CFS at the instant of failure. The surcharge storage lost as a result of this breach would be about 36 acre-feet. From rating curves plotted for cross sections 67 and 68 of the Winooski River near the Montpelier City line, there is 12 acre-feet of active storage available per 845-foot reach length to attenuate this additional flood flow resulting from the breach formation. From flood routing, this peak will decay approximately 30 percent per 845 foot reach so that there will be little difference between the flood stages for the flood before and after the case of the dam failing with the water level at the top of dam. In the first 3000-foot reach of the river where most of the flood attenuation takes place the flood will be within the river banks inundating an area not to exceed 300 feet in width.

SECTION 6 - STRUCTURAL STABILITY

6.1 Evaluation of Structural Stability

a. <u>Visual</u> Observations

The visual inspection did not disclose any findings which would indicate instability of the foundation of the structures.

b. Design and Construction Data

The available design and construction data are insufficient to formally evaluate the stability of the dam. However, the height of the concrete weir is known at two cross sections, as shown on Exhibit L of the Green Mountain Power license application dated June 30, 1966. At the higher of these two cross sections the weir is about 20 feet high from crest to bedrock along the centerline. In the field it was observed that the upstream side of the weir has been filled in with river silt almost up to the crest, in the vicinity of the right end of the weir. Further, Mr. DeForge of Green Mountain Power Corporation has indicated that overtopping of the right abutment has occurred, which means that the water level has been at least 10 feet above the crest of the weir.

Given the cross sections of the weir as shown on Exhibit L and the above loading conditions, and further assuming that the tailwater is 10 feet above the base, the stability of the weir against overturning and horizontal sliding is questionable. Therefore, it is important that the stability of this weir be evaluated in detail if the dam will continue to impound water.

c. Operating Records

None of the available records indicate that foundation stability problems have developed since the dam was constructed in 1908.

d. Post-Construction Changes

There are no known post-construction changes with the exception of the access road reconstruction and the silting-in behind the spillway weir which were mentioned previously in Sections 6.1b and 3.1 b and c.

e. Seismic Stability

The dam is in Seismic Zone 2 and in accordance with recommended Phase I guidelines does not warrant seismic analysis.

SECTION 7 - ASSESSMENT, RECOMMENDATIONS/ REMEDIAL MEASURES

7.1 Dam Assessment

a. Condition

This dam is considered to be in poor condition because the character of the materials in the embankment to the right of the gate house are in part susceptible to internal erosion and in part unknown. Also, the combination of a heavy silt load on the upstream side of the spillway and high water levels that apparently have been reached in the past indicate that the stability of this weir against overturning and horizontal sliding is questionable.

The actual concrete in the dam and abutment structures has undergone significant deterioration. Abundant cracking and spalling are visible along most of the dam and right abutment. In addition, the internal concrete, visible inside the waste gate passageway, has developed flow channels (see Photo 8).

b. Adequacy of Information

The data available were sufficient to bring out potential defects in this structure but are not sufficient to evaluate the seriousness of those potential defects. The Phase I inspection was based principally on visual observations, verbal history and two drawings submitted in 1966 by Green Mountain Power for license application and engineering judgment.

c. Urgency

The recommendations given in Section 7.2 should be carried out within one year after receipt of this report.

d. Need for Additional Investigation

The additional investigations described in Section 7.2 should be carried out.

7.2 Recommendations

An engineer qualified in the design of dams should be engaged to:

1. Investigate the stability of the concrete overflow weir and make recommendations relative to the need for alterations.

2. Investigate the composition and stability, particularly against internal erosion, of the embankment to the right of the gate house, including the entire cross section upstream to the start of the bend in the river.

7.3 Remedial Measures

a. Operating and Maintenance Procedures

- The waste gate and penstock gates should be opened permanently to maintain lower upstream water levels during normal flows. Prior to opening either gate, the Vermont Division of Water Resources should be contacted to specify necessary measures to control the release of silt build-up.
- 2. The forebay cover should be renovated to prevent unauthorized access and hazard to trespassers.
- 3. The gate house should be disassembled and removed from the site.
- 4. The trees that are growing on the embankment to the right and upstream from the gate house should be cut annually.
- 5. The downstream slope of the embankment to the right of the gate house should be kept free of debris and trees so that any seepage through the embankment will be observable.
- 6. All deteriorated concrete at the dam and abutments must either be repaired or replaced if the dam is to remain in use.
- 7. A program of annual periodic technical inspections should be instituted.

7.4 Alternatives

As an alternative to carrying out the recommendations in Section 7.2 above, the dam could be breached to eliminate any potential flood hazards of this dam. Hazards to trespassers should also be eliminated if the dam is breached. However, the dam's capacity to retain ice and prevent severe ice jams in the City of Montpelier should be evaluated before a decision is reached to breach the structure. In addition, the Vermont Division of Water Resources will require proper controls to prohibit the sudden release of siltation deposits downstream.

APPENDIX A

VISUAL INSPECTION CHECK LIST

VISUAL INSPECTION CHECK LIST PARTY ORGANIZATION

PROJECT MONTPELIER No. 4 DAM			DATE November 21, 1978		
				WEATHER 70	% cloudy, 30° F at 12:40 U.S. DN.S.
PARTY					
1J	ames H. Maynes	D-H	6		
2. M	orris J. Root	D-H	7		
3R	obert E. Dufresne	D-H	8		
4S	teve J. Poulos	GEI			
5K	enneth Hadd	GMP	10		
	PROJECT FEATURE			INSPECTED B	Y REMARKS
1					
2		•			
4					
5				···	
8				· · · · · · · · · · · · · · · · · · ·	
9					
10					

PROJECT Montpelier No. 4 Dam	DATE November 21, 1978				
PROJECT FEATURE	NAME S. J. Poulos				
DISCIPLINE Geotechnical	NAME				
AREA EVALUATED	CONDITION				
DAM EMBANKMENT (Earth fill zone to right of right abutment)					
Crest Elevation	618.25 USGS				
Current Pool Elevation	610.25 USGS				
Maximum Impoundment to Date	Foundation of concrete dam-spillway is bedrock				
Surface Cracks	None observed				
Pavement Condition	None				
Movement or Settlement of Crest	Not observable				
Lateral Movement	Not observable				
Vertical Alignment	Not.observable				
Horizontal Alignment	Not observable				
Condition at Abutment and at Concrete Structures	Left abutment - over wash has scoured soit to bedrock on downstream side and has scoured rock. Right abutment - no seepage at concrete bedrock interface.				
Indications of Movement of Struc- tural Items on Slopes	None				
Trespassing on Slopes	Locked gate gives access. There seems to be no trespassing.				
Sloughing or Erosion of Slopes or Abutments	During a past flood river has flowed over embankment to right of right abutment. Probably also dredge spoil has been placed to repair access road to gate house and to widen embankment. Fill is silt and miscellaneous wood trash.				
Rock Slope Protection - Riprap Failures	Upstream face on right side is thick concrete wall; on left, channel is in bedrock.				
Unusual Movement or Cracking at or Near Toe	None observed.				
Unusual Embankment or Downstream Seepage	A small stream (∠3GPM) is present down- stream of right abutment contact with natural ground. Not known whether this is seepage from river or a natural stream. No other scepage observed downstream.				
Piping or Boils	None observed.				

A-3

PROJECT Montpelier No. 4 Dam DATE November 21, 1978 PROJECT FEATURE NAME S. J. Poulos DISCIPLINE Geotechnical NAME AREA EVALUATED CONDITION DAM EMBANKMENT (Earth fill zone to right of right abutment) Foundation Drainage Features None Toe Drains None Instrumentation System None Fully forested with trees on downstream Vegetation side up to about 16" diameter.

PROJECT Montpelier No. 4 Dam	DATE November 21, 1978				
PROJECT FEATURE	NAME				
DISCIPLINE	NAME				
AREA EVALUATED	CONDITION				
OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE	The comments below refer to the intake channel just upstream from the gate hous intake screens.				
a. Approach Channel Slope Conditions Bottom Conditions	The intake screens are almost totally silted in with river silt that has deposited just upstream. Not visible.				
Rock Slides or Falls	None				
Log Boom	None				
Debris					
Condition of Concrete Lining	None - river is approach				
Drains or Weep Holes	None				
b. Intake Structure					
Condition of Concrete	Poor - spalled and cracked				
Stop Logs and Slots	Trash screen - covered by silt at intake to penstock. 1 - outlet - flood gate. 1 - outlet - penstock. 2 inlet screens.				

PROJECT Montpelier No. 4 Dam	DATE November 21, 1978					
PROJECT FEATURE	NAME					
DISCIPLINE	NAME					
DIOUTIAND						
AREA EVALUATED	CONDITION					
OUTLET WORKS - CONTROL TOWER						
a. Concrete and Structural						
General Condition	Poor - dangerous.					
Condition of Joints	Too irregular to judge.					
Spalling Visible Reinforcing	Much visible evidence on all concrete structures. Wire mesh only.					
Rusting or Staining of Concrete	Along cracks.					
Any seepage or Efflorescence	Some seepage in flood gate conduit through concrete.					
Joint Alignment	Seems to be OK					
Unusual Seepage or Leaks in Gate Chamber	Gates are closed tightly. Significant seepage through flow channels in penstock					
Cracks	sluice gate. Several small cracks.					
Rusting or Corrosion of Structural Steel	None.					
b. Mechanical and Electrical						
Air Vents	None.					
Float Wells	None.					
Crane Hoist	None.					
Elevator	None.					
Hydraulic System	None.					
Service Gates	1 - rack and pinion on flood gate (7' x 10')					
	1 - gear and screw on penstock (7' diameter pipe) sluice gate					
Emergency Gates						
Lightning Protection System	None.					
Emergency Power System	None.					
Wiring and Lighting System	None.					

PROJECT Montpelier No. 4 Dam	DATE November 21, 1978
PROJECT FEATURE	NAME
DISCIPLINE	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - TRANSITION AND CONDUIT - EMERGENCY FLOOD GAT	
General Condition of Concrete	Poor,
Rust or Staining on Concrete	Most of the concrete is stained.
Spalling	Large chunks falling off.
Erosion or Cavitation	Severe erosion.
Cracking	The concrete on all three outlet works sho
Alignment of Monoliths	prolific cracking. Too irregular to judge.
Alignment of Joints	Too irregular to judge.
Numbering of Monoliths	Too irregular to judge.

PROJECT FEATURE NAME DISCIPLINE NAME AREA EVALUATED CONDITION OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL General Condition of Concrete Rust or Staining " " Spalling " " Erosion or Cavitation " " Visible Reinforcing " " Any Seepage or Efflorescence " Condition at Joints " Drain Holes Channel Natural bedrock. Plenty of loose rock and trees but form no impediments. Hydraulically OK.	PROJECT Montpelier No. 4 Dam	DATE November 21, 1978	
AREA EVALUATED CONDITION OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL General Condition of Concrete Rust or Staining Spalling Erosion or Cavitation Visible Reinforcing Any Seepage or Efflorescence Condition at Joints Drain Holes Channel Loose Rock or Trees Overhanging Channel CONDITION See Conduit " " " " " " " " " " " " " " " " " " "		NAME	
OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL General Condition of Concrete Rust or Staining Spalling Erosion or Cavitation Visible Reinforcing Any Seepage or Efflorescence Condition at Joints Drain Holes Channel Loose Rock or Trees Overhanging Channel CONDITION See Conduit " " " " " " " " " " " " " " " " " " "		W 1/2	•
AND OUTLET CHANNEL General Condition of Concrete Rust or Staining Spalling Erosion or Cavitation Visible Reinforcing Any Seepage or Efflorescence Condition at Joints Drain Holes Channel None Natural bedrock. Plenty of loose rock and trees but form no impediments.	AREA EVALUATED	ì	
Rust or Staining Spalling Erosion or Cavitation Visible Reinforcing Any Seepage or Efflorescence Condition at Joints Drain Holes Channel Loose Rock or Trees Overhanging Channel """ Natural bedrock. Plenty of loose rock and trees but form no impediments.			
	General Condition of Concrete Rust or Staining Spalling Erosion or Cavitation Visible Reinforcing Any Seepage or Efflorescence Condition at Joints Drain Holes Channel Loose Rock or Trees Overhanging Channel	" " " " " " None Natural bedrock. Plenty of loose rock and trees but form no impediments.	

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APPENDIX C

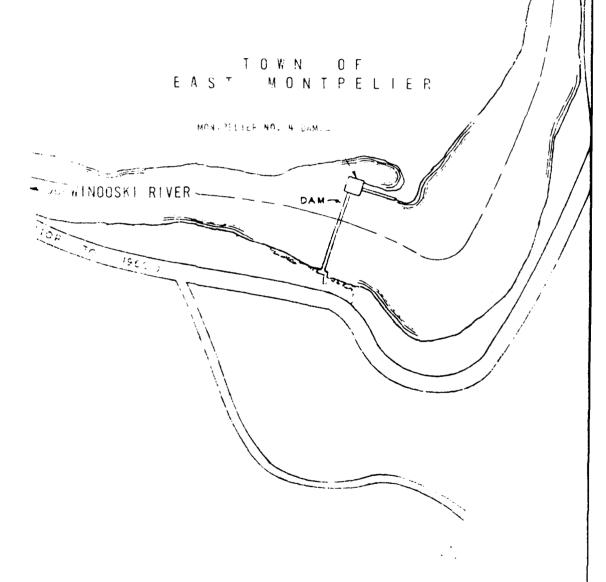
PHOTOGRAPHS AND LOCATION KEY

- 1. View of Gate House and Severed Penstock
- 2. View of Dam Spillway
- 3. View of Left Abutment and Dam Spillway
- 4. View of Right Side of Gate House
- 5. Downstream View of Gate House, Penstock and Waste Gate Passageway
- 6. View of Spalling at Right Downstream Abutment
- 7. View of Severed Penstock
- 8. View of Waste Gate Passageway
- 9. Downstream View of Left Abutment
- 10. View of Silt Deposit Upstream of Waste Gate
- 11. View of Bedrock on Left Side of Impoundment
- 12. View of Siltation on Right Side of Impoundment
- 13. View of Spillway Crest

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TOWN OF BARRE TOVNOF PERLIN FIGURE 3 THE RESIDENCE OF THE STATE OF T MATTOMA PROBLAN OF INSPECTION OF NON-FEE, DAWS MONTPELIER NO 4 DAM CLIENT NO. C4 DO96 FROM SCALE NO. CAR RED GMPC DATE

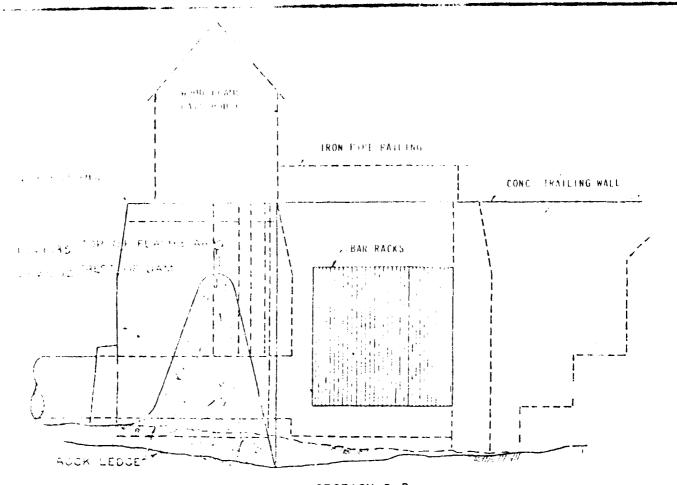
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SECTION B-B
SCALE: 1" 10" APPROX

FIGURE 2

DUFRESNE-HENRY ENGINEERING CORP. U.S. ARMY ENGINEER DIV. NEW ENGLAND

CORPS OF ENLINEERS

NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS

MONTPELIER NO. 4 DAM

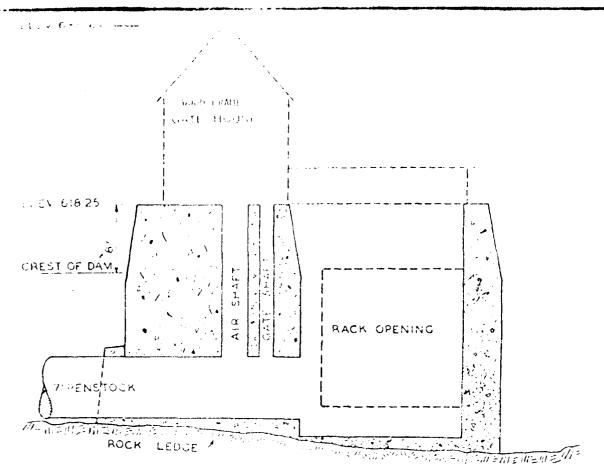
EAST MONTPELIER

VERMONT

CLIENT NO. 04 - 0096 FROM: EXB. L SCALE AS SHOW ENUR RED GMPC DATE 12-19-78

Fire LAN GEF FIGURE 1

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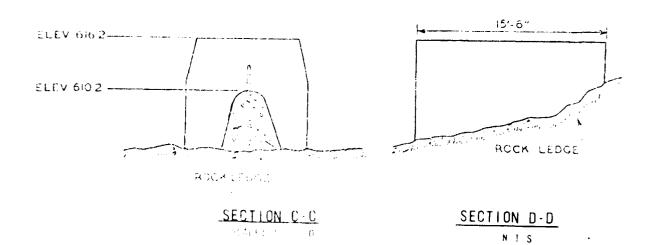
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ELEV 613.

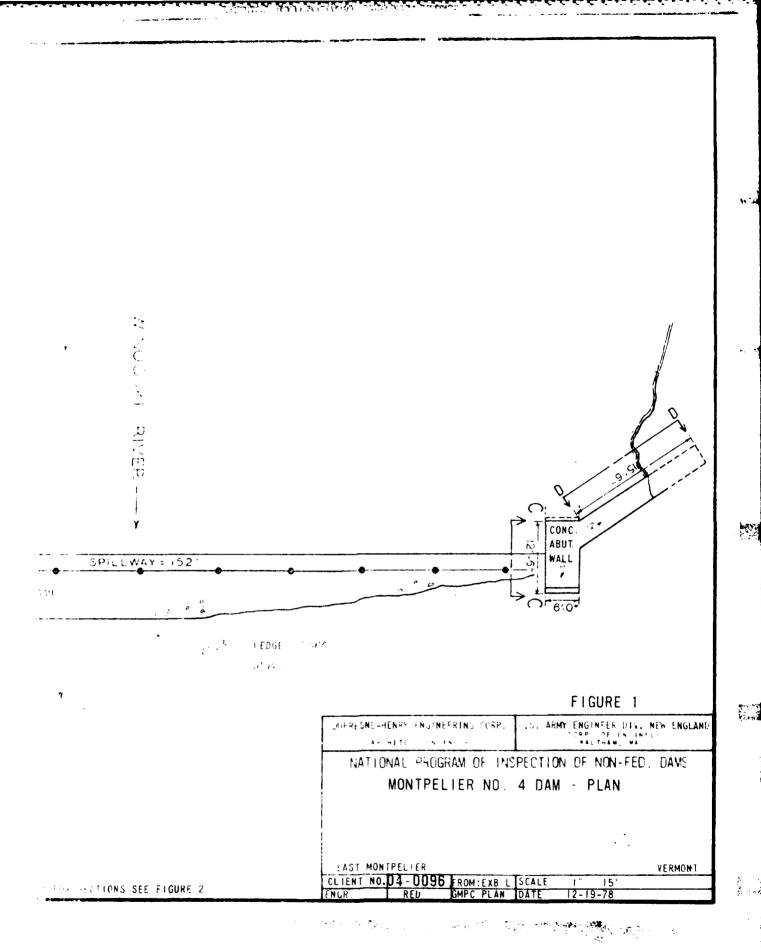
ELEV. 610

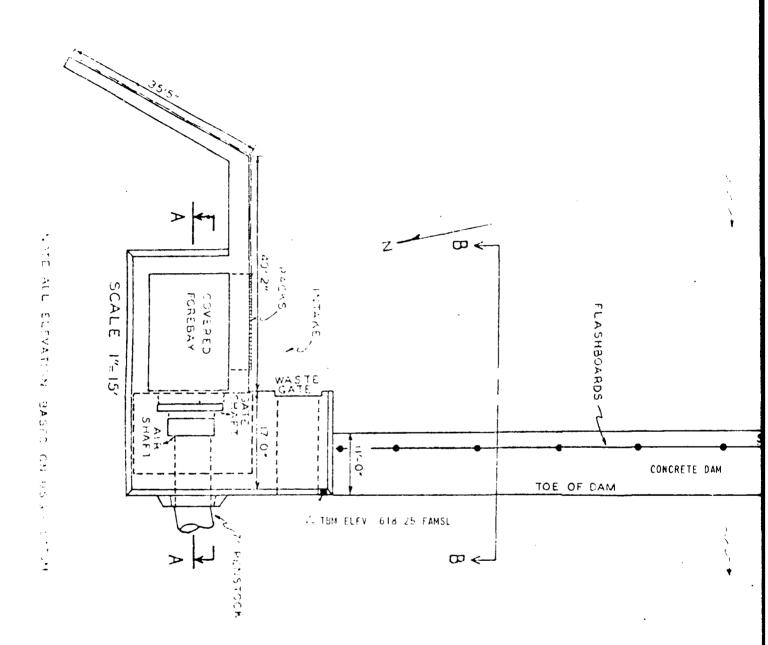
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SECTION A-A



NOTE: FOR PLAN SEE





NOTE: FOR SEC

FIGI

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but for the existing weakened condtions undue damage to the structure is possible.

Conclusions:

According to the writer's observation of this dam, surface disintegration has reduced its section and is further reducing the structure to incipient failure. As indicated herein the intake section is in the poorest condition.

Because of the small storage capacity, under ordinary conditions damage due to a dam failure would be limited.

Nevertheless, rehabilitation work is needed for this dam and should be accomplished soon.

STEPHEN H. HAYPROCK
HYDRAULIC ENGINEER

Public Service Commission Montpelier, Vermont December 9, 1949

Report No. 102

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has sustained a surface failure, i. e., surface scaling and disintegration.

The spillway section, shown in Figure 1 has not been badly affected on its downstream face. The condition is more pronounced on the upstream face, especially at the upper elevation.

As shown in Figure 2, the intake section is in the worst condition. Besides the loss in section due to disintegration there is leakage in a number of places around its perimeter. Because of the leakage, alternate cycles of freezing and thawing can further disintegrate the concrete mass.

Miscellaneous Comments:

No major repairs have been made to this dam since its construction. Because of the almost complete silt conditions in the reservoir, the owner is skeptical about the relative worth of rehabilitating the structure.

In reviewing the discharge capacity of this dan it

was found to be inadequate as far as the maximum probable flood is concerned. Ordinarily, the dam could stand some overtopping,





Dens. P.

REPORT ON MONTPELIER #4 DAM OF THE GREEN MOUNTAIN POMER CORPORATION

Reproduced from best available copy.

Introduction:

Montpelier #4 dam is one of the developments on the Wincoski River belonging to the Green Mountain Power Corporation. It is located at the Berlin, East Montpelier town line about 3 miles upstream from the city of Montpelier, Vermont. Built in 1908, it is operated in conjunction with a low head hydro-electric plant.

The dam creates a pond having a surface area of about 5 acres at flashboard height. Under present silt conditions, the useable volume impounded by the dam is about 700,000 cu. ft. The drainage area is about 201 sq. mi.

General Description:

This dam is described in detail in Barker & Wheeler's 1926 appraisal report of the property, then operated by the defunct Montpelier & Barre Light and Power Company. In general, it consists of the following:

A spillway section, about 152 ft. long is built on ledge rock and spanning the river channel. At the maximum section it is about 21 ft. high, 3 ft. wide at the crest and 19 ft. wide at the base. Both faces are sloped. The spillway crest is 6 ft. below the top of the dam. Flashboards are provided on the crest to a height of 2.5 ft.

North of the spillway, and adjacent to it, is an intake section containing the usual complement of trash racks

APPENDIX B

PROJECT RECORDS AND PLANS

PROJECT Montpelier No. 4 Dam	DATE November 21, 1978
PROJECT FEATURE	NAME
DISCIPLINE	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - SERVICE BRIDGE	Not Applicable
a. Super Structure	
Bearings	
Anchor Bolts	
Bridge Seat	
Longitudinal Members	
Underside of Deck	
Secondary Bracing	
Deck	
Drainage System	
Railings	·
Expansion Joints	
Paint	
b. Abutment & Piers	
General Condition of Concrete	
Alignment of Abutment	·
Approach to Bridge	•
Condition of Seat & Backwall	

PRO	JECT <u>Montpelier No. 4 Dam</u>	DATE November 21, 1978					
PRO	JECT FEATURE	NAME					
DIS	CIPLINE	NAME					
	AREA EVALUATED	CONDITION					
	LET WORKS - SPILLWAY WEIR, PPROACH AND DISCHARGE CHANNELS						
a.	Approach Channel	·					
	General Condition	River - mostly open with some silt.					
	Loose Rock Overhanging Channel	None					
	Trees Overhanging Channel	None					
	Floor of Approach Channel	River bottom - bedrock not visible, may be silted behind spillway.					
b.	Weir and Training Walls						
0.	General Condition of Concrete	Dam appears to be poor - irregularity of slope to spillway. Concrete poured on original ground as left abutment/ground.					
	Rust or Staining	None					
	Spalling	Spalling on downstream and at left training wall.					
	Any Visible Reinforcing	None					
	Any Seepage or Efflorescence	Not visible.					
	Drain Holes	Not visible.					
c.	Discharge Channel						
	General Condition	ОК					
	Loose Rock Overhanging Channel	None					
	Trees Overhanging Channel	Lined with forested slopes.					
	Floor of Channel	River channel bedrock.					
	Other Obstructions	None					
		1					

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DUFRESNE-HENRY ENGINEERING COR

ARCHITECT-ENGINEER

NATIONAL PROGRAM OF

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PHOTO NO. AND VIEW DIRECTION

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Esta and DUFRESNE-HENRY ENGINEERING CORP. U.S. ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASS. NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS **PHOTOGRAPH** LOCATION KEY MONTPELIER NO. 4 PHOTO NO. AND VIEW DIRECTION F. MONTPELIER VERMONT SCALE NTS

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1. VIEW OF GATE HOUSE AND SEVERED PENSTOCK



2. VIEW OF DAM SPILLWAY



3. VIEW OF LEFT ABUTMENT AND DAM SPILLWAY



4. VIEW OF RIGHT SIDE OF GATE HOUSE



5. DOWNSTREAM VIEW OF GATE HOUSE, PENSTOCK AND WASTE GATE PASSAGEWAY



6. VIEW OF SPALLING AT RIGHT DOWNSTREAM ABUTMENT



7. VIEW OF SEVERED PENSTOCK



8. VIEW OF WASTE GATE PASSAGEWAY



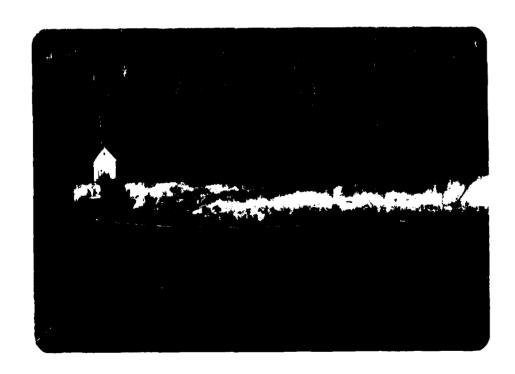
9. DOWNSTREAM VIEW OF LEFT ABUTMENT

10. VIEW OF SILT DEPOSIT UPSTREAM OF WASTE GATE





11. VIEW OF BEDROCK ON LEFT SIDE OF IMPOUNDMENT



12. VIEW OF SILTATION ON RIGHT SIDE OF IMPOUNDMENT



13. VIEW OF SPILLWAY CREST

APPENDIX D HYDROLOGIC AND HYDRAULIC COMPUTATIONS

DUFRESNE-HENRY ENGINEERING CORPORATION

BY	SUBJECT	SHEET NO.	OF
DATE		JOB NO	

Appendix D

Hydrologic / Hydraulic Computations

Test Flood D-1

Sterage Routing D-1

Rating Curve D-2

Discharge Capacity Comps. D-3

Failure Analysis D-4

HEC-2 For Dom Failure D-11-1-00

Dramage Area Map

DUFRESNE-HENRY ENGINEERING CORPORATION

BY	SUBJECT	SHEET NO. D_ OF
		JOB NO 4-0096
DATE		JOB 110:

Tost Flood

Hazard! failure poses no threat to life > Low

Low Hazard, Small size 50 to 100-yr frequency

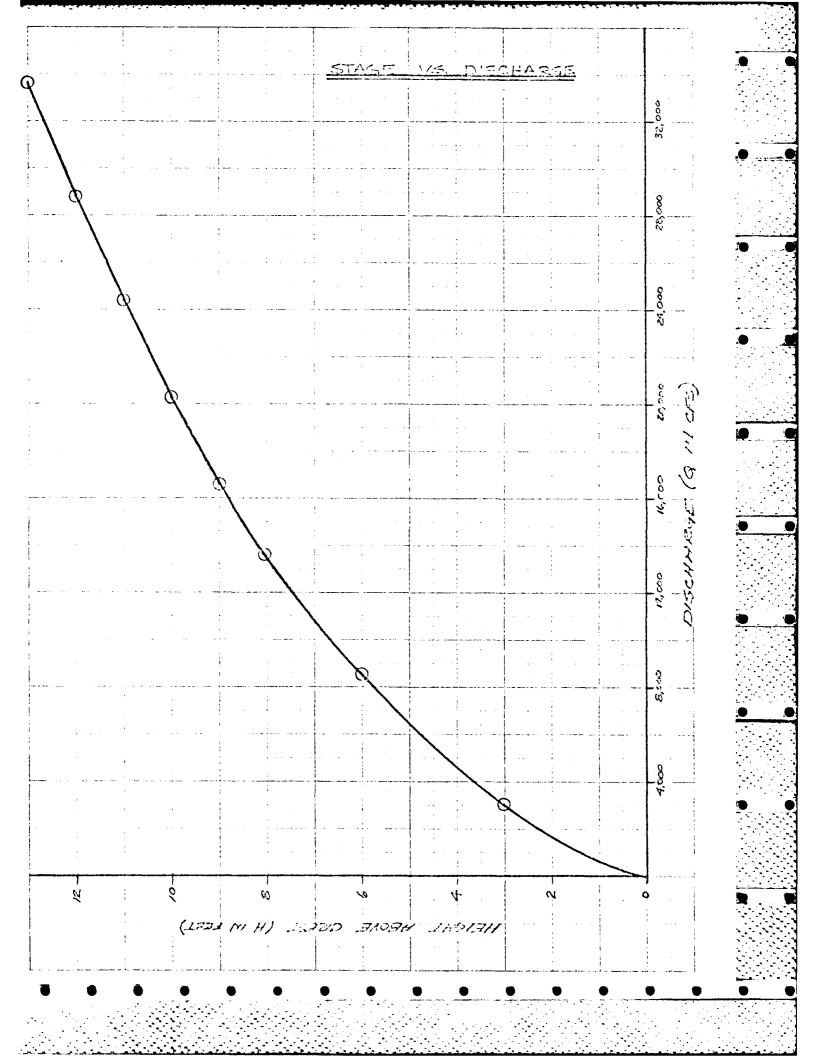
From Flood Insurance Study for City of Montpelier, flow a Windowski Friver above Stevens Branch => 17,000 cfs for Q100.

: Storage Reating

Pont Surface Area = 4-5 Acres

Drainage Area = 201 square miles

Storage is "run-of-river"; therefore, nouting would produce insignificant reduction peak flood flows. No capacity curves are developed or routing performed. Test Flood = 17,000 cts



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DUFRESNE-HENRY ENGINEERING CORPORATION
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BY MATERIAL SUBJECT AND SHEET NO. D-3 OF DISCHARGE CALACITY JOB NO. 110000

West 1820 Re 01/2/2 1207 00001 610,2 -1714 1814 HAVE 6162

2711 11311 North 612,05

b= 3' 4 = 3/28 (52)/2 = 3/064 fs 11119TH 152

ASSUME C FOR DAM = 3.83

h= 6' Q= 2.33 (62) (5) = 8.6835.

h= 8.05' ිකු ව පැවැත්තේ දිහිට් වේ වනුවන + Z.5(21.5)(5.2) = 158 Q= 13,603

g = 3.35 (152) 3) = 15,934 100

* z.e (21.6)(3)² 279 + 2.e (185)(.95)² 417

16,620 Q = 16,620 cfs

h= 10' G = 3.85 (102)(10) 1/2 - 18,650 + 2.5 (21.5)(4) 430

+ 2.0 (1841)(1.01) = 1235

Q = 20,305 of 3 20305

h= 11 Q = 3.88(152)(1) 1/2 - 21,516

2.5 (2.15)(5)3'= 601

2.5 (150)(200)3/2 = 2280 Q= 24,307 cf.

24,397

Q = 300 (102 X 10) 1/2 = 1: 12' 24,516 2.5 (21.5)(6)22 = 1190

25 (137)(202) =

Q = 28,839 ofs 28,839

Q = 3.60(10)(16) = -1 13' 27,643 m: (m.d)(n)3/2 -935

+ 2.5 (197)(4050) + 4956

33594 Q= 33,591.4-

CHESAST	PRINTOUT	6.0	Maria Tiers	PROFILES

MINOCSKE N	. 84. TC TC:	4										
	MARMEL MEN E Ength - Reach		EL OF		GISCHARGE BCFS<	CHSEL	CALME	EG	TOPHLA	101-2	TIME	val
212.35	0.0	0.0	٥.٥	534.8	0 10540.00	513.24	513-24	540.33	130 -1	91.40	4.3	0.0
212.45	0.6	0.4	ن . ن	5-4-6	(31()	517.71	364.14	525.10	140.74	15.02	0.0	4.0
212.05	0.0	3.0	۵.0	5.4.4	i jjecter	5244 52	523.52	524. 74	150000	21.45		V- 0
212.05	0.0	0.0	0.0	5-4-4	48100.03	525.22	525.42	531.57	3-4-51	52.34	0.0	J_8
222.08	110.60	0.0	J. C	541.4	2 11346.00	516.32	٠.٠	51 a. 14	445.47	1.62	0.61	3.51
222.31	113.00	3.3	J.C	931.6	C Block.co	525.28	3.0	326.47	166.00	8.45	0.00	4.72
222.17	113.63	4.3	٥.٥	5-1-4	3 31030.03	5.4.19	0.0	527-46	186	4	پ ز ہال	1.01
222.01	110.03	3.3	J.J	\$	C 48104.LO	530.42	٠.٠	532.24	240. **	19.50	J. JJ	4.34
232.09	103.00	ں ر ر	3.3	533.0	J 10546.60	514.25	4.0	516.42	157. 14	10.43	0.31	7.73
232.39	10).00	J. 0	٥.٥	501.0	3 31000.00	544.46	0.0	326.44	111.10	12.35	J.EL	44.54
212.09	133.63	9.0).3	503.0	G 3300J	575.84	4.4	341.41	102.06	12.52	7 - 4 1	43-45
232.09	100.00	9.0	0.0	533.6	9 48765.60	534.15	0.0	512.45	422.30	12-42	7.71	50.11
242.10	63.00	0.0	0.4	534.1	0 10540.03	515.79	J. 6	517-10	154.76	35.45	0.61	9.44
242.10	43.C3	0 .3	J.3	536.2	3 31600000	324.44	3.0	526.43	1/4.01	22.76	3.04	10.40
242.10	63.60	4.3	٠. ن	536.7	J \$3046.63	545.33	4.4	521.52	177.01	22.34	0.01	2v. 34
242-10	60.00	0.0	٠.٠	اسدو	3 401.3	324.31	٥.٥	312-74	175.10	22-14	3.01	25.40
24.10	10.60	514.10	5-1-53	324.5	3 10946.03	514.09	٥.٥	518.83	1 . 7	40.01	J.01	9.50
24.10	10.00	514.13	330.53	5-8-5	\$ 11. adada	524.25	ن ۵۰	513.40	\$13.44	32.01	0.01	14.45
. 24.10	13.0	514.10	528.50	500.5	3 33693.00	521.48	0.0	551.33	3/4.04	32.03	3-31	24.05
24-10	13.33	314.10	304.33	304.3	3 48100,53	531,74	u. 6	334042	453.00	33.70	0.01	20-67
252-11	10.00	0.3	٥. ن	534.5	3 13540103	514.72	3.0	516.74	144.00	70.36	0.04	10-11
252.11	10	ა. ა	J.3	504.5	3 21000.00	528.87	٠.٠	530.45	100.01	14.73	0.01	44. / 6
252.11	Lu.co	J.3	0.0	5.4.	3 33600.00	344.18	ندن	531.49	310.40	21-64	1.01	20.11
452-11	10.00	0.0	9.0	501.5	3 40103.03	\$30.45	0.0	254.58	401.44	14.66	0.01	27.54
262-12	100.00	J. J	4.0	511.6	0 10540.00	518.74	564-74	521.12	152.46	76.11	3.01	12.00
202.17	100.00	ن د و	ن ۽ ن	511.4	J 31600.00	\$21.10	0.4	531.35	1+1-2>	38.19	J. Ú.	25.10
242.12	133.00	0.3	٥.٥	911.0	i Bicurica	521.12	 .	532.23	141.45	44.06	0.04	26.54
262-12	103.03	4.4	٠. ن	314.4	. 48/34.04	316.31	230.21	5 34 . +3	344.23	51.72	0.41	34.52
26.30	41.00	\$1;.90	524.43	511.4	u 10540.34	519.23	0.0	521.78	133.04	12.40	0.0≥	12.15
24.30	41.65	533.73	976.80	511-6	10 31L, J. CJ	528.98	٠.٠	512.21	149.24	26.07	0.01	27.34
24.13	41.00	911.40	524.84	511.0	10 11:00.00	524.95	J. Q	512.77	161.10	34.20	4.01	44.55
24.10	41.03	221.40	526,80	511.4	3 44100.00	530.61	0.0	210.41	354.11	47.74	u.ut	11.00
272.15	130.CC	J.3	0.0	513.4	C 10540.03	521.39	u.,	522.49	161.20	29.31	0.02	25.64
214.15	132.00	0.0	J.J	543.4	3 31000.03	311.01	0.0	532.63	291.03	13-65	3.01	15.54
212.15	133.00	J. U	J. G	511.4	0 11603.30	531.47	0.0	553.25	305.53	14.13	J.01	36.90
272.15	133.00	0.0	3.0	513.2	48700.00	\$35.55	U-U	537.66	111.45	15-76	0-91	41-54
202.17	100.00	0.0	٥.٥	513.4	12 12545.20	521.11	4.0	522.N	175.99	29.32	4.02	10.21
202.17	100.03	J. 3	J. J		3 11063.00	511.00	0.0	352.44	345.00	13.89	9.02	43.44
202-17	100.00	0.0	0.0		13 33663.60	512.25	U = U	\$13.44	375.05	11-14	0.02	45.74
267 17		0.0	4.4		AL ARROW GO	514.52	4-0	313.45		4.17	0.01	Au 14

					•						
x t	4.923	97.600	1381.000	1461.033	543.000	543.000	543.000	U. 0	J. Q	V.0	
Ĝ	543.430	0.0	343.233	50.000	571.560	156.600	552.900	2120000	544.500	365.443	
68	945.432	473.033	565.130	4/3.000	546-663	500.003	540.200	514.000	541.203	663.000	
64	544.400	674.433	3-6.433	1,,,,,,,	544.340	#24.EUG	551.200	\$ 00 a 200	554.500	420.000	
GR	336.000	1030.000	334.4.4	1225.006	553. 960	12/5.000	554.500	131>	544.103	13/5.010	-
64	333.400	1381,0.3	> >	1101.000	215.100	1340.600	5 35 .2 24	1407.000	934.400	1414.003	
69	5 14. 800	1424.0.0	535.200	1434.400	53500	1452.600	536.000	1461.000	530.400	1464.003	
68	244.440	1473.003	334.500	14/3.313	553.400	1445.003	\$52.430	1523.001	555.000	10-5-663	
GR	\$50.000	18-5-663	\$47.500	1444-033	544.400	1740.000	540.600	1407-000	545.900	2015.000	
68	563.630	2313.333	5+6-133	2315-000	445.200	2340.663	544.830	2043.600	343.403	2044.000	
GR	545.200	2110,000	944.2.3	2100-400	569.100	2210.000	551.200	2250.000	551.800	2330.003	
68	\$51.400	2350.630	510.133	2450.003	C. C	4.0	7.0	J. 0	0.0	J.U	
11	4.923	C.0	J. u	0.0	1.60	1.000	1.000	4.0	0.0	0.0	
ŝī	2.000	1375.010	554.500	544.100	1410.000	554.540	544.100	. 4.0	8 _ 0	G. 0	
	444.930	0.0	0.3	٥.٥	35.000	35.000	35.000	4.0	0.0	4.0	
W.	0.0	3.0	0.5	4.0	0.0	0.6	1.000	4. 0	0.0	0.0	
				4.0	1.000	1.000	1.030	u. a	0-0	0.0	***
£ L	4.910	0.0	3.3	0.035	1280.600	0.013	1400.000	·. a	0.0	8.6	
-	1.010	0.673	1101.000	0.033	0.500	0.0	3.0	v.0	0.3	0.0	
#C	10 س	4.013	3.615	0.0	0.,00	•••	***	***			
23	455.010	31.000	1234.333	1257.000	524.000	524.603	528.000		0.3	923 <u>~</u> 03	
64	\$71.930	٥.٥	572.443	45.000	5.8.200	219.600	345.404	120.000	545-100	554.000	
68	\$40.000	350.633	540.233	424.333	542.2.0	513.6.0	\$44.400	524.000	546-400	#4 i. cui	
GR	\$40.320	674.303	551.230	133.630	554.500	11	550.000	# 14 a 2000	340.500	1161.003	
64	540.136	844.033	3+1.111	1111	334-6-3	1040.600	341.700	1120.000	549.200	1215.003	
64	\$14.1.0	1204.633	5 50.000	1221.000	340.500	1251.000	543.700	1240.000	\$62,300	1370.000	
58	\$62.100	1547"013	951.333	1304-030	331.430	1110.00	3:1. #30	1130.000	3.3	0.0	
64	\$70.310	1500.003	٠.١	0.0	0. 0	0.0	٠.٠	u. u	9.0	9.0	
RC	0.036	0.653	1.115	3.0	G. 500	U.0	3.0	4.0	•••	4.0	•
11	665-19C	17.037	521.430	639.000	£45.600	445.660	445.044	٠.٥	0.0	9.0	
6A	944.600	2.0	591.100	33.000	551.300	300000	220.074	345.000	596. 600	434.000	-
64	\$54.033	411.033	332.630	921.000	541.000	533.600	341.000	605.000	947.600	a /\$	
64	\$50.100	#45.023	592.403	#25.000	560.600	455.000	500.000	463.600	344-600	421-623	
GR	361.610	1415.0.0	500.533	1441.000	u. :	ų.u	٠.٠	4.0	3.0	0.0	
N.C	0.090	4.043	3.035	0,0	0.500	3.6	0.0	٠. ٥	•	V.0	
X1	4/3.310	11.000	270.000	343.090		845.600	#65.000	u. 0	0.0	0.0	
64	\$40.410	0.0	501.303	51.000	504.640	230.0.4	564.100	210.000	550.100	300.003	
64	540.730	112.003	592,700	300.000	554. 100	314.000	363.534	ن د د د د د د	540.400	4/9.665	
GA	\$14.500	5-1-633	U.J	J.U	٠.٠	4.0	ي. ن	444	v. c	0.0	-
NC	0.050	9.043	0.40	0.0	U. 500	0.0	J. 0	4.0	3.0	u. c	
a t	485.510	19.003	440.333	510.033	445. 640	#45.600	8+5.000	v. 0	0.0	0.0	
Ğ.	396.010	0.0	343.000	33.333	544.8.0	10.000	332.400	44.040	5=0.400	2 50.000	
64	9/11.043	493.333		212.000	5/1.740	340-000	511.400	110.003	\$74.4.0	333.000	
GR	565.6.0	359.033		413.003	554.000	464.644	553.004	514.044	\$10.000	560.000	
ĞÂ	\$15.600			365.000	383.600	575.000	545.600	*42.443	4.6	٠.٠	
- 63	0.0	0.0	٠.٠	J.0	٥.٠	0.0	0.0	4. 4	0. 0	0.0	

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-								 0	0.0	6.0	
MC	4.036	0.090	0.015	0.100	0.200	٥.٥	0.0	u. y	5.0		λ .
X1	5/4-190	15.000	58.000	103.070	525.000	525.000	525.000	J.0	0.0	4.0	;
ç.	514.000	0.0	354.700	0.106	557. #CU	72.003	515.000	5000	529.630	58.000	-
5.0	523.600	78.000	521-000	102.030	524.400	100.000	514.903	172.000	536.000	331-000	H :
ŭ.	\$16.100	306.030	337.034	363.030	540.600	165.100	354-034	100-200	5 10.000	365.300	E.S.
								_		0-0	1
A L	544.270	14.633	100-110	144-000	457.000	420.000	.20.000	4.0	U.G 545.0UU	41.000	
44	310.000	0.û	516-000	3.138	558.003	14-100	553.400	\$1#_UUU	526.000	125.000	-
- 2	596.800	91.000	520.003	71.000	524.630	100,000 2,3,000	525.600	310.000	534.890	423.833	. 1
6.0	526.000	154.073	523.333	200.003	535.600 550.600	420.300	310.000	420.400	0.0	0.0	_ :
u.B.	335.600	426-133	143.330	473,303	0,200	0.0	0.0	4.0	0.0	9.0	;
NC.	3-450	0.043	٠, ١, ١	3.133	0.7.70	***	•••) i
#1	394.343	30.000	وراد ـ مادو	434,303	470.000	.70.000	470.000	u. 0	0.0	0.0	
G.S	510.000	3.0	363.330	100000	560.600	30.000	555.500		555.500	45.000	·
. 4	544.000	144.033	533.633	195.003	517.400	284.630	524.200	324.000	524.800	345.000	:
64	3/6./00	186-043	524.430	437.00	527.563	434.000	5 4 1 . 3 00	407.000	210.834	519.000	-
54	313.64U	505.003	550.233	655.03	519.4.0	155.000	537.000	**	5-0-000	949.000	
CR	5.4	1649.633	342.443	14.7.034	345,600	1003.066	950.000	1612.000	555.000	1635.000	· · · · · · · · · · · · · · · · · · ·
64	555.000	10.5.013	122.000	Landouid	400,003	1805.563	365.040	1612-030	510-000	1,50-004	- :
u?	4.336	16940.033	11,,,,,,,,,,	146	34164.643	9.0	4.6	4.0	٧.٥	0.0	
#i	0.000	6.043	3.343	٠.٠	0.300	0.0	٥.٥	u. 0	J.0	0.0	•
						1320.000	10/0.550	u. .0	0.0	0.0	-
21	4.548	19.000	290.000	340.000	1050.600	20.606	356.000	68.000	345,040	78.000	
GR	3/5.413	40.633	510.000	14.000	345.000	138.643	344.400	420.000	526.340	230.000	•
ű ä	\$ 00.230		343.400	201.000	341.200	25	\$23.400	254.000	525.600	241.000	1
us Us	326.603	215.033	524.100	281	511.143	284.000	534.600	240.000	554.440	301.000	· · · · · · · · · · · · · · · · · · ·
6.8	313.900	300.000	511.300	314.000	313.100	342.633	\$44.400	324.000	544.000	193.003	ŧ
60	303.000	1014.533	302.200	1157.033	345.660	1504.600	\$54.400	1570.000	\$55.000	1514.030	<u>ن</u> ي ا
Ca	355.000	1.44.033	333.403	41.400	363,604	1834.600	\$65.000	1800-000	\$70.000	1 444.000	•
•-											- }
I 1	4.345	0.0	3.3	3.3	1,000	1.000	1.000	v. 0	0.0	0.0	I.
81	2.300	224.000	344.410	531.403	324- (00	544.410	5 34.400	0 منه	0.0	0.0	
									0.0	9.0	
31	616.556	0.0	٠.٠	0.0	20.600	53.00	20.000	u. 0	0.0	0.0	1
A č	0.0	9.0	3.0	9.0	0.6	4.9	1.000	u. 0	•••	***	1
		9.0	0.4	0.0	1.000	1.604	1,000	u.0	4.0	0-0	
41	0.453	U. U40	0.0	3.5	0.550	0.0	0.0	u.0	4.0	9.0	ş
MC	5.030	0.053	113.000	0.033	250,600	C. G+0	110.000	w. 035	672.COJ	0.040	
Na-	1912.140	C. U	3.3	J. 0	0.6	U. 0	0.0	u. 0	0.0	0.0	ų
•	14.1	•••	•••	•••	•••	***					
3 i	6/4.710	\$1.003	782.333	847.000	450.330	950.000	950,000	u. 0	0.0	0.0	
تَمَ	\$10.000	0.0	965.000		Sac. Cou	35.000	155,444	50.000	550.000	65.000	
(4	393.243	19.010	5-0-003	از ال ال و د ال غ	515,000	115.633	5:3.00	154.000	524.463	183.000	
61	528.500	201.033	5 . 1 . 3 . 0	214.6.2	534.600	254.630	\$ \$ 1,000	315.000	537.230	500.00 0	
5.1	537.414	300-0-3	510.500	9/4.657	537.8-0	655.GOU	937.763	140.000 844.600	535.900	850.000	
64	\$34.000	110.000	528.873	165.000	521.446	482.000	3.0.000	1130.000	544.800	1430.000	
51	\$34.530	81520	234+410	944.003	540.600	1000.000	5 44 . LJJ 52 LJ I I I	140.000	349.300	1519.000	
0.0	545.436	13.9.003	500.433	1317.000	5 to . 6 W	0.0	0.0	J. 0	0.0	0.0	·
CA	544.700	155+-000	543.000	1677.003		0.0	4.4	•••	•••		
	. 34 . 020	40.033	#J4.J33	441.333	474.000	476.963	476.000	U. U	0.0	0.0	į
41	983,020	93.033	363.030	13.006	386.600	44.003	555,000		552.000	103.000	
	30.000	131.003	503.444	151000	3-3.100	110.000	537.100	101.000	534.400	221.000	
64	534.000	225.000	310.070	144.000	310.100	504.600	5 31,000	513.000	531.000	317.000	
2.4	312.736	5/3.674	539.133	5/4.003	515.300	645.444	\$30.500	15,.000	فدن و وو	163.000	
-1	337.000	833.434	531.+33	855.000	531.530	400.000	511.700	960.000	530.100	565-000	
فَن	417.000	1625.000	531.100	1005.001	\$ 35.460	10+5-000	\$ 50.000	1105.000	344.749	1165.000	
64	343.743	1275-904	540.330	1344.006	546.600	1305.000	547.500	1394,000	5.7.500	1510.600	
<u>,</u> •	544.630	1454.013	340.613	140,	345.503	1500.003	543.400	1902-000	349.200	1675.000	
Ç.P	5-5.800	1435.033	\$44.930	4543.434	345. 6 00	1541.003	345.200	1845.000	574.700	1745.000	
Ç.A	504.130		551.230	1705.003	351.800	1825.600	9.0	u. J	. 0.0		

GR.	520.300	444.030	533.603	444.003	534.200	443.000	539.400	904.000	545.600	514-000	11
GR.	346.430	342.000	543.233	551.633	510.000	567.640	0.0	4.0	0.0	0.0	' !
NG.	0.050	0.650	3.345	4.133	0-100	u.a	0.0	u. 9	0.0	0.0	
										0.0	
R à	40.352	18.000	274.000	336.000	540.000	540.000	340.000		0.0		8
GA .	514.003	0.0	553.630	5.100	550.466	14.604	344.0.0	40.000	545.000	86.000	• •
	542.430	143.000	524.200	162.333	524.200	140.680	326.2W	222.000	252.104	214-000	
S.R.	525.430	116.033	512.400	334-0-2	534.000	334.606	5 9 3 . 0 0 0	441.000	542.407	444-000	1
	544.433	440.000	334.634	502.603	5/3.000	652.000	0.0	w. U	٥.٥	0.0	
NE.	0.050	0.050	411.0	0.100	0.200	0.0	0.0	u. 0	u. 0	0.0	
~~	0.070	0.0.0	4,427	••••		•••					• •
a t	513.652	17.630	244.330	264.000	663.000	480.000	6 20.000	J.0	3.0	0.0	'
			354.633	22.003	154.000	75.CG0	341.600	25.100	544.443	104.000	
٠	510.000	3.0		12.000	524.400	111.000	545.410	244.000	926.403	254.000	· į
ÇB	5,1.000	134-027	\$15.000			100.000	310.000	360.000	544. aud	3/3.000	
(A	520.030	303.313	324.833	322.000	534.400				0.0	9.0	~ .
G	344.400	403.033	210.333	445.606	0.6		0.0	 0	0.0	•••	
				341.603	140.000	790.000	740.060	u.o	0.0	0.0	,
A 1	523.800	\$5.000	441.333	24.003	351.000	34.000	333.000	71.003	222.200	115.000	- ;
C.	910.000	0.0	900.000			171.000	516.400	391.000	934.200	111.400	
ų ą	251.200	136.003	945.133	141.000	3.3.600	353.444	5/4.00	310.000	932.433	5/4.000	
ű.	215.000	445.000	250.000	441	522.800				555.000	441.000	
LA	541.600	991.633	545.333	611.133	552.000	491-003	225-200	*******		9.0	
68	364.035	614.033	510.030	081.100	4.0	u. Ģ	U. U	4.0	0.0		
NC.	0.400	J. 143	0.015	0.136	0.200	0.0	0.0	ه چن	4.0	0.6	
			\$20,000	.500_000	440.000	600.000		J.0	0.0	0.0	
z i	533.020	24.000				160.060	598.000	214.000	540.600	236.000	,
61	5/3.000	3.0	563.333	34.332	344.400	440.000	344.000	4%4.000	939.00	544.004	• ;
	545.244	264.000	502.030	114.000	543.200			340.000	524.400	632.000	
G.R	333.200	518.730	529.200	250-0-0	324-2-0	332.003	526.430			712.000	
GR	578.600	4.3.003	533.230	453.303	245-000	490.603	942.000	104.000	\$44.000		• •
Ç.R	954.434	715.033	555.030	123.000	560.000	*30.660	514.000	Inc. cod	4.4	U. 0	<u> </u>
**	0.090	0.013	0.335	0.100	0.233	c.c	٠.٠	v- 0	0.0	0.0	`
									9.0	0.0	!
*1	31.100	11.033	201.130	120.000	\$40.000	540.000	340.000	0.0 194.000	540.330	144.100	š.
C.	5/4.530	3.	,,,,,,	1.6-1	500.000	160.00	544.830				
64	534.6.0	269.000	525.323	233.125	524.600	229.000	252-011	214.633	252-109	324.030	
.4	525.700	342.033	334.030	5	540.603	371.000	544.800	321-177	5+4.404	5-3.033	2
Ç.	554.000	500-110	513.333	900.200	0.0	0.6	6.0	٠.٥	4.0	0- 0	1
						5.000	5.000	w. 0	0.0	0.0	
x t	33.200	0.0	0.3	0.0	5.000					0.0	į.
# T	2.033	130.103	564.430	543.200	351.030	544.400	560.200	4. 0	4.0	U.#	ŗ
	53,100	0.0	٠. ٠	3.0	15.600	19.000	15.030	u. a	0.3	0.0	=
4 L 82	33.100	0.0	9.9	8.0	0.6	6.6	1.600	w. 0	0.0	0.0	. •
44	4.5	0.0	4.0	•••	***	•••					
x t	514.020	0.0	3.0	0.0	1.000	1.060	1.400	u. 3	0.0	G. C	
								_			
AL	34.480	16.033	145.400	230.000	100.000	100.00	100.000	. v. 0	0.0	0.0 104.000 '	· · · · · · · · · · · · · · · · · · ·
4 ن	914.333	J.3	\$43.233	تدورن	500.600	102.063	530.000	14.100	334.400		
GR	530.000	1.0.103	321.344	144.300	525.000	102.400	324.500	2 was 600	251.073	230.000	•
مي	918.900	211.100	334.414	2+2.000	300.600	200.600	344.000	250.030	398.924	34-000	
ũa.	\$70.000	342.160	١.٠	0.0	0.0	0.0	٠.٠	0	0.0	0.0	
								_		4.0	1
21	34.200	0.0	0.9	٥.٠	5,000	5.000	3.000	u.0	0.0	0.0	
. 7	2.000	101-100	344.000	539.400	3+2-630	\$**.000	539.400	w. 9	0.0	4.0	ı i
	34.300	0.0	٥.٥	0.0	25.000	25.000	25.000	w. 0	0.0	0.0	
Ħ	0.0	0.0	0.8	0.0	0.0	6.0	1.000	J. 6	0.0	4.0	
~	4.0	V.0	V. T	•••							1
41	554.050	0.0	9.0	0.0	1.660	1.000	1.000	u. 0	0.0	0.0	
# L	544.290	19.030	245.330	320.000	211.000	211.660	211.633	J. 0	0.0	0.0	
دء	5/3.300	J. 6	363.330	10.203	542.200	10.500	501.200	42.000	542.003	43.000	
ان	340.433	103.0.0	510.633	100.000	524,543	1/5.000	521.400	221.000	526.600	255.000	
Ğ.	520.000	245.313	529.033	118.000	543.833	320.600	502.000	331.033	545.600	144.603	
Ģ=	20 0 - 400	207.733	,,,,,,,				4 ** 0.10	41 14.0	0.0	Δ.Δ	

2.3	13.000	C. 0	0.0	J.0	0.0	0.6	U. 0	u-0	0.4	0.0	
G.B	560.000	0.0	543.300	91.000	537.430	140.000	536.000	164.000	535.400		
64	515.940	208.633	>>>.200	325.000	>>2.60C	342.000	52,633			190.000	·
GF.	313.440	500.000	520.400	363.033				445.000	515.800	504.000	
Ğ.	334.100	434.003			526.200	\$10.000	\$ 12.600	500.000	536.600	542.00 0	• 1
6.8			327.000	842.433	527.934	616.000	520.200	100.003	532.500	734.000	
	333.230	755.033	211.470	893.603	538.360	450.000	340.000	920.100	545.000	920.200	
Ç.A	\$60.000	923.363	4.5	0.0	0.0	e.e	0.0	w.G	0.0	0.0	
41	451.090	14.033	63.000	155.000	130.000	430.000	1 10.000	٥.٥	9.0	0.0	
68	\$10.000	J. U	553.400	0.100	511.600	0.200	533.000	** 000	527.400	4.100	•
ü٩	522.230	32.033	\$11.033	61.000	311.400	153.000	522.200	140-000	525.800	204.100	1
La	512.700	311.633	512.100	221.000	550.000	221.100	570.000	221.230	0.0	0.0	
										•••	-
41	45.100	29.003	71.100	131.020	5.000	9.600	5.000	u.0	0.0	4.0	4
Y 5	٠.٥	i.o	0.3	535.233	530.603	6.0	0.0	J.0	0.0	0.0	
C.E	\$10.000	0.c	550.330	0.403	513.000	4.200	533.000	4.000	527.430	4.100	
	517.400	61.003	515.243	01.010	535.200	71.010	511.600	74-1-0	514.600	112.000	
ان	917.430	141.633	915.700	142.010	535.260	153.040	\$17.000	154.020	522,200	140-000	
ÇA	>4>.00	200.033	332.730	211.000	534.100	221.000	550.000	221.100	\$10.000	241.200	•
11	45.200	0.0						_			
îż	U.0	0.0	J.J	3.0 333.230	25.000	25.000	25.000	U. B	0.0	0.4 4.0	
							0.0	•••	***	1.0	
#1	45.300	0.0	0.0	0.0	1.000	1.000	1.000	٥.٥	0.0	0.0	
MC	0.040	3.043	0.335	0.140	0.200	0.0	0.0	4.0	0.0	0.0	· -
41	463.193	22.033	661.000	113.033	235.600	215.000	235.000	J. 0	9.0		
64	370.000	2.0	301.300	182-003	543.400	300-606				0.0	
	514.200	*******	\$12.500	*******	532.100		337.300	3 # 4 . 0 0 0	\$37.500	433.000	¥
	310.000	447.033				\$25.000	201-100	34	319.100	•10.000	
54	332.700		510.133	114.000	218-000	135.663	320.400	300.000	254-408	745.000	
		824.033	534.833	855.033	535.000	***.C.U	334.000	94u. UJJ	341.200	413.000	
G.	>4>.000	V89.003	373.333	1610.000	0.0	0.0	J. 0	u. ù	0.0	4.8	š
41	473.250	21.013	428.403	441.000	335.000	555.000	\$55.000	v.a	0.0	0.0	!
• ن	510.000	0.0	551.633	1.633	37/.300	22.463	345,400	40.000	534.000	160.600	l l
ů a	317.834	174.003	515.434	204.000	534.000	310.000	521.233	39.6.424	320.000	349.000	
Ü.	911.935	111.013	314,633	421.603	517.530	459.000	314.400	64	323.034	407.000	· '
C.B	\$14.030	462.C33	535.300	324.003	344.200	314.600	363.200	381.000			1
-	314.4.4	736.633	3.3	3.6	0.0				543,534	415.000	Į.
NC	0.000	2.013	0.315	0.100		U.O	٥.٠	٠.٠	a.a	a.a	
~	0.043	7.013	0.333	0.100	0.300	C.G	٥. ٥	u. u	•.0	0.0	
11	485.290	26.003	110.413	LDD.EVE	234.000	210.0W	230.000	٠. ٥	0.0	0.0	3
Ç	310.303	4.0	342.200	103.003	540.800	167.660	5 40.900	314.000	539.464	310-100	
64	537.400	510-210	515.433	311.053	533.50	041.616	333.300	310.000	130.133	310.100	•
	911.703	314.033	\$20.033	314.10.	510.2.0	337.6.0	347.000	364.600	517.403	373.000	
- 4	522.640	410.000	924-333	******	532.646	**8.100	314.000	449.033	333,400	447.160	
	511.413	412.023	\$14.433	452.103	541.000	491.600	343.000	520.000	347.900	353-060	
GA	270.000	\$60.003	٠.,٠	3.6	ů. L	0.0	747.000	2.0.0	0.0	9.0	
MC.	3.060	0.063	3.435	0.100	U-200	0.0	0.0	Ú. 0	0.0	0.0	
AT	44.100	310.303	\$43.403	9.0 535.540	9.CG2 452.CC0	5.660 539.400	\$.000 334.000	W-0	0.0	0.0	
••		,,,,,,,,	,,,,,,	737.700	432.000	739.400	334.000	J. 0	0.0	0.0	
41	44.200	0.0	0.0	0.0	25.000	25.040	45.400	4.0	0.0	0.8	
113	4.0	0.0	0.0	0.0	3.0	0.0	1.400	w.0	9.0	0.8	
41	48.250	9.0	0_3	0.0	1.00	1.000	1.000	U. 0	4.0	0.0	
40	0.060	0.043	3.015	0.100	0.203	0.4	3.0		0.0	0.0	
81	44.300	0.0			433.646	430 000	4.50.0				
NC.	0.350	0.450	0.0 U.JJS	n . 100	620.00	620.000	420.0W	8.0 U.O	0.0	0.0	-
							V. •	•••		4.4	
Al	443.420	21.033	213.303	549-100	10.000	10.063	10.000	J.D	0.G	0.0	
64	5/3.636	0.0	\$51.000	15.000	950.400	57.603	341.000	140.030	\$44,4GJ	151.000	
	944.236	142-033	5+1.433	550-003	534.400	214.660	336.000	213-000	\$34.000	273-100	

									519.000	234.000	• •
Ç.	512.000	103.003	530.400	144.033	529.200	143.000	519.800	200.000			7.1
G. B.	211-3	222.003	515.633	232.000	515.203	334.003	515.000	340.000	519.000	377.404	1 1
62	524-023	345.033	530.030	343.000	534.464	460.633	530.130	514.003	531.400	556.000	
			510.710	583.000	534.300	632,600	341.000	614.000	941.200	406.COO	
CA	\$11.602	914.000									٠, ١
	541.100	629.011	350.000	#30.630	560.600	846.000	0.0	u. 0	0.0	0.0	_ 13
MC	0.060	0.060	3.435	0.100	0.200	U. U	0.0	u. 0	٥.0	0.0	_ [3]
ε,	482.540	31.000	132.000	336.000	500.000	500.000	500.000	 0	0.0	0.0	, ,
									544.000	41.000	,
	>00-0	1.0	229.970	14.000	550.000	10.000	550.000	30.000			
	915.400	101.000	531.600	111.000	531.200	119.000	511.400	137-000	530.900	149.000	i
i. R	914.930	152.000	521.233	1 . 7	544.500	168.000	544.204	100.000	514.400	208-000	,
٤٠	514.4.0	229.003	515.400	2.0.000	510.500	244.606	519.800	تعادها	523.800	323.000	
			511.400	916.000	510.100	4/1.000	530.300	342.000	532-100	605.303	
Ų.	524.530	110.000									3 4
GE	510.400	114.003	343.33	144.000	544.900	#24.6W	341.440	461.000	553-433	463.000	·
L.A	303-000	943.030	 .	J. U	4.0	J.0	U_ U	4-0	3.3	0.9	***
NC	0.060	0.043	0.015	3.100	0.260	0.0	4.0	u. 0	0.0	0.0	
# 1	392.650	20.033	19.000	291.003	653.000	650.000	650.030	u_0	0.0	0.0	
										74.000	· -
Ç.	340.000	0.0	993-333	3.634	519.000	0.203	533.000	30-000	531-200		
ĩ.	523.100	61.077	521.00	101.030	510.100	112.000	\$10.000	22	521.403	265.040	
CO	524.100	245.030	311-231	2 * 1 . 3 4 3	511.2W	384.000	524.030	414.000	533.700	545.000	
GR	333.940	628.003	>>0.00	713.000	330.400	134.600	334.000	110.103	360.000	734.200	

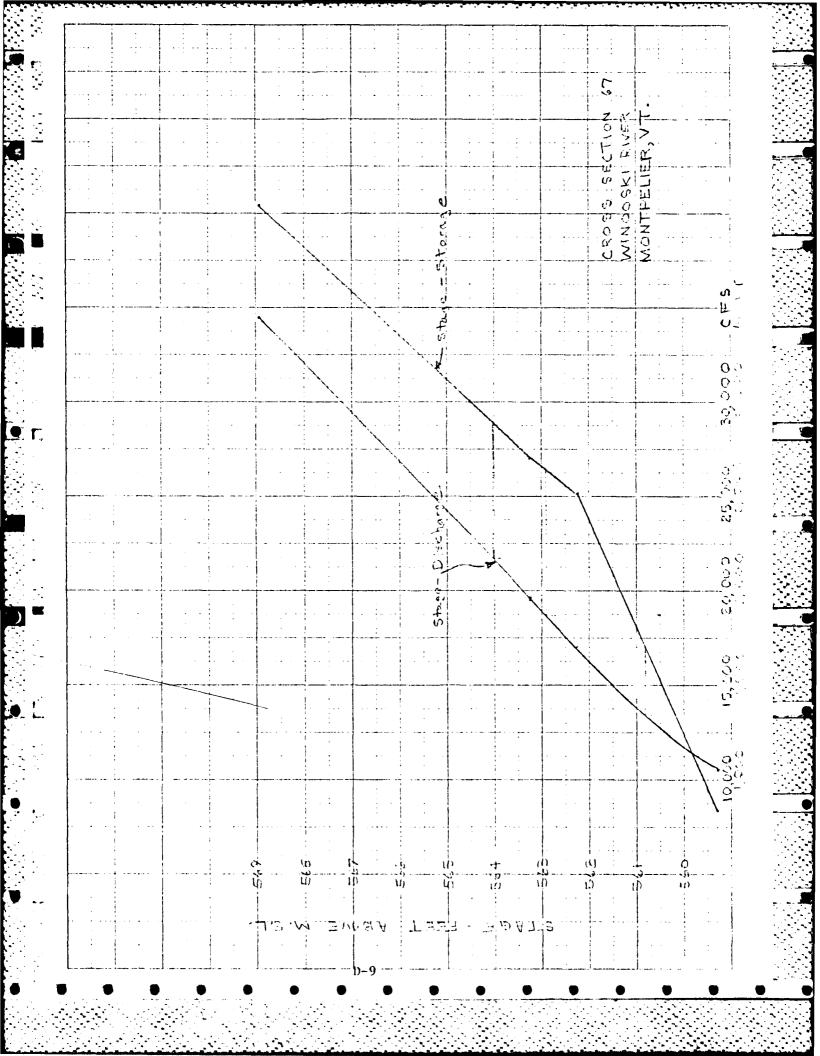
		0.0	J.J	0.0	10.600	10.000	10.000	J. 0	0.0	0.0	
AL	34.173									331.200	
41	4.000	94.000	533.200	533.430	7s. cco	335.200	231.202	293.000	233-500		
b T	324.000	511.210	311.200	0.0	0.0	G.C	0.0	u. 0	0.0	0.0	-
# L	39.200	0.0	0.0	0.0	10.600	10.000	10.000	U. 0	0.0	0.0	•
72	0.0	0.0	0.0	0.0	0.6	0.0	1.000	y. 0	0.0	4.0	;
**	0.0	0.0	0.0	***	***	***	11000	4.4	•••	•••	•
										0.0	1
4.1	39.300	c. c	9.0	0.0	1.000	1.000	1.000	v. 0	0.0	0.0	:
											`
81	402.730	23.000	111.000	244.001	360.000	360.000	360.000	Jan 2	0.0	0.0	
64	950.000	0.0	144.230	28.603	337.000	44.GUG	334.230	80.000	532.400	114.000	
		110.022	923.233	111.000	217.200	174.003	317.400	2022000	\$20.000	249.600	
Ć S	\$\$7.200										
64	524.0.0	357.033	311-533	344.663	312.200	379.003	لىدى. 22 ۋ	414.000	332.300	444.000	
64	531.400	518.630	311.0.0	445.003	533.460	640.CUJ	333,400	*******	536. Juj	111.600	
GA	500-200	111.133	550.003	129.636	560.600	7/4.400	0.5	u. 0	0.0	0.0	i
н.	0.066	J. (L. J	411.0	4.143	8.230	y.a	0.4	u.0	0.0	4.4	
п.	0.000	0.040	0.377	*****	*****	***		•••	***		, ,
									0.0	0.0	-
4.1	417-833	26.013	145. 334	350.630	310.600	51 c. coa	516.600	U.0			
L	300.000	u. 0	343.333	71.660	531.600	45.603	530,800	121.000	533.200	154-000	
4	\$14.000	157.053	933.630	172.663	111.600	181.000	534.000	105.00	5/4.400	148.600	3
Ğı	914.430	210.003	310.403	745.003	517.200	211.603	514.6.0	324.600	525.200	338.000	
	527.500	364.023	534.414	******	5.8.400	454.000	540.400	502.000	\$29.2.3	333.000	
									350.000	849.613	
L.	532.323	909.043	534.533	645.CUC	334.402	987.CAA	\$14.800	447.000			
_8	500.000	150-000	3.4	J.J	6.6	4.0	0.0	u. 0	Ú . 8	0.0	
N.	0.343	0.063	0.415	0.103	0.200	4.0	J.0	u. 0	0.0	0.0	
2.1	422.910	24.000	122.333	269.003	439.6.3	0.05.000	905.000	u_0	0.0	0.0	
					537.6-0	76.000	3 36.200	45.000	530-000	93.000	
	300.000	C-0	334.443	15.533							
GA	176.430	**.033	523.433	105.000	520.000	124.000	310.200	166.033	917.003	216-603	
54	514.810	269.033	520.000	262.003	528.000	354.640	5.4.900	361.000	529.603	411-600	
6.0	931.230		\$34.173	300.603	244.462	000.243	517.000	642.3.23	337.603	PP5-COA	·
فئ	934.433	489.033	100.000	111.011	550.663	322.000	\$60.000	750.030	0.0	0.0	
									0.0	4.0	
MC.	0.163	0.063	3.345	u. Lud	3.200	u. c	u.u	u. u	V. V	0.4	
								_			
41	432.490	24.033	231.000	441.438	425.660	425.600	•25.000	U_0	0.9	0-0	· · · •
قټ	503.030	0.0	551.110	10.033	545.263	44.003	5 1 . 6 23	103	536-100	113.000	
GA	530.100	197.000	533.600	26:	921.603	213.605	>22.030	284.430	517.200	332.000	
	310.100	361.033	\$14.633	437.063	521.600	439.003	\$21.000	447.000	526.400	410.000	• •
										#10.COO	
34	5/6.400	999.003	321.610	575.600	930.200	404.000	935.900	435.000	516.400		
GR	531.303	839.613	*10.411	451.443	544.843	831.000	3 44 . 830	864.000	33C.000	A 1A * POO	
C R	563.030	986.633	J. U	·-•	0.6	0.3	0.0	u. 0	0.0	0.0	•
-											•
			4.10		43. 544	410 401	4 44 444		4.4	0.4	

											الأعلمين أراء بالأما
		143.000	504.530	187.003	509.200	228.600	512.300	440.000	514.800	271.000	
68	510.500			323.300	1527.400	384.000	520.900	4534700	346.800	474.000	
GR	524.000	211.003	528.633	323.000	72	300000	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	***************************************			
				212.200	10.000	10.000	10.000	4.0	0.0	0.0	
#1	252.110	19.003	121.333		530.500	58.000	330.400	70.000	528.000	121.000	
GA	\$60.030	0.0	\$ 10.304	24-030				187.000	\$09.443	227.000	
GR	523.600	151-100	513.500	151-500	510.100	143.003	504.500		524.600	121.000	~
GR	512.430	241.433	514.433	111-033	323.400	212.200	524.000	212-200		4.0	
GR	521.100	380.663	250.477	453.606	250.100	4 74 , 600	>60.000	400.000	0.5	0.0	
											· · · -
X L	242.120	21.033	443.100	564.030	100.00	100,000	10.000	0	0.0	0.0	
GR	560.000	3.0	951	16.000	541.900	51.00	5 38. 400	\$11.330	331.900	244.000	
GR	\$10,200	359.033	532.230	دده.ده	534.404	443.000	330.00	447-100	217.300	دن ند و ۱۹	
68	\$11.700	441.133	511.633	454.000	512.203	504.600	514.300	364.400	514.300	577.000	
GÂ	522.000	505.603	537.373	545.100	529.504	592.640	524.600	457.000	528.300	665.000	
ĞÂ	\$63.330	445.103	3.4	J. 0	0. C	v. 0	J. 0		U. U	٥.0	
33	0.900	1.540	2.5.0	J.0	141.600	3.000	2115.000	u_Q10	511.400	511.60C	
	9.700	,									
		0.0	٠.٠	3.0	41.600	41.000	41.600	u. 0	0.0	0.0	
X 1	24.300		1.110	524.800	231.534	0.6	u. 0	u . d	د. ن	0.0	
2.2	0.0	3.0		J.0	249.600	532.500	J. J	355.000	530.200	J.0	
ĄΤ	11.000	111.033	516.674		531.500	0.0	506.530	532.700	0.0	500.000	
81	404.000	533.203	J. J	443		657.000	328.000	J. U. U	445.000	520.360	
8.7	\$32.300	٥.٥	245.000	254-209	0.0			J. 0	0.0	0.0	
8.T	0.0	485.1C3	214.677	J. U	u.e	6.6	0.0	0.0	***	•••	
									0.0	0.0	
z i	212.650	51.003	1+1.333	123.000	130.000	130.000	130.000	.	529.200	96.000	
64	\$60.000	0.0	550.334	7.163	532.500	0.200	224.990	74.000			
GR	540.440	114,633	254.879	121.343	524.644	Libboots	214-320	144-000	511-1-0	143.000	
64	514.300	112.003	213.233	383.600	513.900	255.000	214.100	201-404	511.200	320.000	
68	519.300	324.000	524.433	311-009	521.500	322.310	524.300	332.100	550.040	132.500	
GR	540.030	332.360	v. J	٥.٥	0.6	C.S	0.0	 0	0.0	8.0	
NC.	3.050	4.450	3.335	J. 10J	0.200	9.0	0.0	u. 0	U.U	0.0	
	••••	••••	*****								
X1	262.170	21.003	45-170	244.003	100.000	100.000	100.330	J. J	0.0	0.0	
ĈŔ	560.000	3.0	550.400	0.100	524.300	6.200	521.000	41.600	527.430	11.640	
64	341.600	#JJJ	324.433	84.003	527.103	¥5.003	340.404	V3.100	520.010	161.000	
	314.100	130.033	311.000	1140	513.400	171.000	263.700	200.000	514.430	244.640	
68		244-000	511.133	240.000	524.400	244.003	\$21,000	44.3.0	990.00	400.200	
G A	310.700			3.4	6.6	4.0	٠,٠		U_U	u.0	
64	564.040	463.763	U.	3.103	9.200	6.0	J. 3	v. u	0.0	0.0	
NC	0.050	0.653	4.015	3.100	81140	***	***	•••	•••	***	
					300.000	300.000	300.000	·- ·	0.0	0.0	
#1	312.239	21.000	133.333	349.000			321.000	••.000	526.400	100.000	
GR	\$60.300	0.0	223-170	0.103	526.500	0.200			514.200	104.003	
64	\$20.340	115.000	\$20.130	1900000	250-103	13/.063	284.103	141.000	314.100	312.400	
GR	\$14,200	231.603	514.600	250.003	515.360	105.00	510.000			485.100	
GR	320.700	345.033	251.470	100.000	221.230	3#5.030	521. 300	443.430	550.000		_
68	560.000	485.240	3.3	J.C	U. L	0.0	u.a	u • 4	0.4	0.0	
RC.	4.700	3.043	4.335	3.133	3.233	0.6	u. a	J.0	0.0	0.4	
										- 4	
	352.310	21.003	114.000	324.202	400.000	440.000	•00.000	u.0	0.0	0.0	
Ğ.	364,000	0.0	524.433	11.000	528.140	34.600	521.640	******	321.600	107.009	
68	540.440	114.333	525.433	124.503	564.303	131.000	514.000	183.000	212-979	120-000	-
GR	516.000	281.033	511.400	324.006	519.100	324.100	526.200	254-500	251.477	154.100	
GR	544.000	373.0:3	524.543	442.003	>24.844	404.000	321.033	45+.333	\$29.200	922.000	
ĞÂ.	512.100	393.013	512.500	021.000	500,630	641.14	٠,٠	₩.0	4.4	ناون	-
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	\$62.340	21.003	141.000	300.000	433.600	430.000	410.000	u. 0	0.0	0.0	
X L			550.000	25.000	393.144	36.600	311.900	80.000	532.100	97.000	
GR	300,000	4.6	521.140	154.003	324.300	1/3.6.0	524.100	131.330	527.200	147.000	•
GR	524.4.0	145.003		211.000	515.444	234.000	545.003	211.000	545.500	161.000	
GR	320.000	239.033	914.330		519.700	357.000	527.500	341.000	544.400	455.000	
GA	\$15.500	324-034	214.330	144.000			534.000	604.300	303.300	654.000	
GA	230.100	646.013	20,200	223.000	231.102	316.000		0.0	4.4	9.0	
GR	\$50.000	111.025	201-113	161.003	٥.٠	6.6	4.0		0.0	0.0	
NC	0.460	3,063	3.035	3.133	0.200	u.a	u.0	w. 0	4.4	~, ~	
										0.0	
K L	372.440	26.000	\$34.303	111.000	303.000	300.000	304.000		0.0	174-007	
44	844-000	4.0	544-430	73.030	937.600	81. (0)	534.500	10000	534.400	144.007	

MECZ VERSION UPOATEU JAN 1975 Barda Corrections Uliuziusiaaiusiusiaaiofius Moutelcatiuns Szinsainaiiniintiis

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	1.000	0.0	J. J	a.a c.e	c.a	0.0	0.4	0.0	0.0		
91 4C	4.0JG 9.050	16544.00U 0.650	31000,000	35633,003	48200.000	0.0	v.0	0.0 0.0		4.0	
4.1	212.050	86.033	178,000	4.2.036	J. 0	0.0	Ç. 9			٥.د	
GA	343.000	0.0	524.433	ل د ن ن ن و	52#. 400	42.443	520.10			154.000	
68	311-103	154-100		114.003	534.942	166.443	244.00			ينعب ۽ لنه لا	
64	340.634	511,000		185.0)4	\$23.400	300.000	524.00			454-000	
CR	560.000	421.000		3.6		u. 0	4	y. (د د ن	
M.C	u.u45	0.345	0.355	0.109	0.200	L.3	٠.٠	0. 3	4.3	0.0	_
21	222.070	23.003	136.033	241.000	110.000	113.650	110.00	o	J.0	٠ ن.د	
68	\$00.000	0.0	553,444	4.1.3	529.100	1.440	264.60			44.000	
ĞA	\$ 10.900	142.030	525.030	131.000	300.400	114.000	3-1.4-	U 419.5		192.200	
64	304.000	159.063	503.00	Lie.G.C	5 .6. 4	241.600	901.90	·· · · · ·	1-4 5-5-513	204.000	
64	\$ ua . UJO	324.000		34.9	930.8.00	314.107	233.00			1,4-170	
NC	0.050	0.053	3,355	3.403	3.300		٠.٠	٠.٠		U.4	
	232.090	23.004	131.333	257.000	100.000	100.000	100.00			J . u	
GR	463.000		953.323	7.140	5 10.000	6-000	510.0.			64 09	
GR	5 10.100			41.101	524.360	110.000	500.00			110.200	
64	500.913			111-0-3	203.900	144-000	561.00			437-049	
6.	\$30.030			283.443	124.500	244.603	541.00			4/0-444	
6A	\$26.500			*****	500.6.0	441.000	0	U . 1		0 - 0 0 - · 1	
NC	0.050	0. 05 1	3.435	3-193	0.100	0.4	c.0			•.•	
#1	247.100	14.000	120.033	271.003	63.000	80.000	40.00	U	0.0	9.0	
GR	560.000		227-179	3.103	530	6-0-3	593.44			64.500	
GR	510.100			111.000	512.500	114.603	911.60			144.00	
64	538.730			293.003	507.600	240.062	șul.a.			a 11.000	
GR	\$10.100			481.436	390-644	481.100	Sayade			u. 0	
NC				3-100	0.200	J. U	4.3			w. d	
50	0.0	0.0	2.433	1.0	445.600	0.461	4_4	, L	314.103	114.100	
# L	24.100		122.100	415.303	10.000	10.000	10.00			0.0	
12		0.0	تان ن و د	204.277	514.160	ŭ. 0	ب و ب	۔ ن		9.0	
45				٠.٥	3.4	0.0	0.0	٠.		4.0	
87				0.0	51.400	530.403	0.0			9.9	
81				122.103	\$14.100	ن د د	143.0			141.000	
81		344.00		514.100	340.440	328-000	214.4			720.00	
87				545.000	521.700	378.200	3.0			3	
					311.100	/#.004	5/4.0			122.103	

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DUFRESNE-HENRY ENGINEERING CORPORATION

rial No. 2 Parameters of (13,400 + 5300 = 18,700 cfs)

stage storage Active storage = 22-12 = 10 Ac-ft \times 5-68 570.6 2662 \times 5-67 562.9 \times 640 \times Avg = (10+13)/2 = 11.5

22 20-6+

 $Q_{p} = 8,300 \left(1 - \frac{11.5}{36}\right) = 5,650 \text{ cfs}$

9, 10 = 13, 4 cro + 5, 650 = 19, 050 cfs

49 < 2% .. use Qp2

Tonclude: Ofature = 8,300 cfs at 36 Ac-ft will be reduced expreximately 32% por 845 foot reach subject to asymptotic decay. Therefore,

Q at first highway bridge, U.S. Route 2, (x5 69+65)

I mile downstream of dam will be

very close to & prior to failure, 13400 cfs.

Q at first location for overbank Slooding, 3000

feet do not com of dom, will also be about the

same as that proof to factore. The impact of

the down halting is mind by comparison to the flows

conditions which would exist prior to the dam failing.

The Solved when this dist 3,000 feet reach will be

within broken boundating as once not to exceed 300

fact in midth.

DUFRESNE-HENRY ENGINEERING CORPORATION

BY M. ROOT SUBJECT Dam Falore France SHEET NO. D-6 OF DATE 4/21/27 for water at "Top" of dam JOB NO. 34-0094

Donz Failure Enlader at top of donz - concrete spillway faling

Crest of dam = 610.2

length of spillway = 1521

which of water to be of top of clan = 13,400 cfs

Discharge in remaining dam = CLH3/2 = 3.88 (9/2)(8) = 3000 cfs

is horse through breach = 8/ w 75 312 = 8/ (6,18)(5.37)(262) = 12700 cfs

The state of the second decrease due to hadre = 21,700 cfs = 21,700 cfs = 1000 cfs = 21,700 cfs = 21,700 cfs = 1000 cfs = 21,700 cfs =

Hydraule Conactly of Breach = 9 = (3.05)(60.8). Hole = 13,400 ets

$$H = \frac{(13.400)^{2/3}}{(187.26)} = 17.2'$$

Stage wehind down drops to (592 + 17.2) = 607.23'
Volume Ketaned of instant of tribic = 40-9 (7) = 12 Ac-ft

Volume Released = 48-12 = 36 Ac-St Volume Released (5)

Ster - Routing relay 1. HEC-2 out out

Stage 1 1/4 of Matpelier 7/2 XS 6.8 for 21,700 cfs = 571.5 X 5 07 for 21,700 cfs = 564.0

17.700 cfs 17.700 cfs 17.700 cfs 18.70 cfs 568.5 1(87) 56.4.0 2905 560.8 1675 25 Ac-ft 14.1 12 Ac-ft

Difference (Active to Stod-wave rading) = 25-12= 13 Ac-ft

 $Q_{P_2} = (7, (1 - \frac{\sqrt{3}}{5}) = 8,300 (1 - \frac{13}{36}) = 5300 \text{ cfs}$ D-7

DUFRESNE-HENRY ENGINEERING CORPORATION

BY K.A. Legitus	SUBJECT MONTPELLER #4 DAM	SHEET NO. D- 5 OF
DATE 75 19	EXHEMPTIC FOR OF CARSO,	JOB NO. Gt DOORS

100 YE 1200 COUNTIONS

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ري	NISMA).	CORRESPONS - DAM FAILURE		10,570 ds
	100 yes	FLOODS CONSTITUTIONS	Baren ann gerannele market	31,000 ofs
	Kar yes	LIGOD MUH ENEVER LEGION	-	33, 600 cfs
	ion ye.	FLOO WITH TANK FAMORE	Barrier Street, Street	AR, 1700 ofs

| Normal Comparisons - Ward Friend - 19,520 of 100 yr theory with the truend -- 19,600 of 100 yr theory with the Think thouse -- 19,600 of 100 yr theory with the Think thouse -- 22,720 of

SUBJECT OF THE SUBJECT OF THE SHEET NO. D. A OF JOHN STREET NO. D. A OF JOHN S	1///	UFRESNE-HENRY ENGINE	ERING CORPORA	TION
Miser = 9.1 + 610.2 = 610.3 Miser = 9.1 + 610.2 = 610.3 Office of Miser	DATE - 7/6///	SUBJECT A MATTER AS THE SUBJECT AS T	Walysis	SHEET NO. 1 OF
Mose = 9.1 + 610.2 = 610.3 The Description of Day Free Description of the Day Free Description of			·	
Mose = 9.1 + 610.2 = 610.3 The Description of Day Free Description of the Day Free Description of				
Mose = 9.1 + 610.2 = 610.3 The Description of Day Free Description of the Day Free Description of	16th year 1 the said	er mind in a) / '	
The Total Francisco willow Property Colored And Fauls $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19519 \text{ ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19610 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19610 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19610 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19610 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19610 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(2)^{2/2} = 19620 \text{ down to ds}$ $G_i = \sqrt[4]{152}(6.27)(6$				
G= 9/21 WLV9 1/2 = 19,510 ds G= 9/21 (14/52)(5.51)(2)/2 = 19,510 ds M = 5/511 (14/52)(5.51)(2)/2 = 19,510 ds M = 5/511 (14/52)(5.51)(2)/2 = 34151 df M = 5/511 EVILLE INDERS MOVE FILLD COUNTY ONE: Change = 9/21 WLVG 1/3/2 G= 9/21 (20)(5.51)(31)/3/2 = 19,7/69/2 G= 9/21 (20)(5.51)(31)/3/2 = 9,827 efs Grand = 17,000 + 5827 = 22,827 efs FOU THOUS FILLD FRILDS WOUR FOUND COUNTY ONE:	West =	9.1 + 610.2	= 619.3	<u>*</u>
G= 9/21 WLV9 1/2 = 19,510 ds G= 9/21 (14/52)(5.51)(2)/2 = 19,510 ds M = 5/511 (14/52)(5.51)(2)/2 = 19,510 ds M = 5/511 (14/52)(5.51)(2)/2 = 34151 df M = 5/511 EVILLE INDERS MOVE FILLD COUNTY ONE: Change = 9/21 WLVG 1/3/2 G= 9/21 (20)(5.51)(31)/3/2 = 19,7/69/2 G= 9/21 (20)(5.51)(31)/3/2 = 9,827 efs Grand = 17,000 + 5827 = 22,827 efs FOU THOUS FILLD FRILDS WOUR FOUND COUNTY ONE:				
G= 9/21 WLV9 /22 = 10512 fs R= 1/21 (.4)(52)(5.47)(2) = 10512 fs R= 1/21 (.4)(52)(5.47)(2) = 2/212 fs R= 1/21 (.4)(52)(5.47)(2) = 2/212 fs R= 1/21 (.4)(52)(5.47)(1.1) = 1/2,7/6 fs G= 1/21 (.2)(5.57)(3(1)) = 3,827 fs G= 1/21 (.2)(5.57)(3(1)) = 2,827 fs	1000 1000 1000 1000 1000 1000 1000 100	<u> </u>	D ASSO	$\frac{12.101/0}{10}$
The court of DAM FAILING $E = R_{e} = \frac{1}{1} \frac{1}{1}$	Sie Sterr WL	12 / 2	32 (1327)	
EVENTURE FINALE NUMBER 100 JEN FLOOD CONDITIONS: (PROPER = 927 WAVE STATE = 17,7% of s $G_{7} = 927 (4) (50) (50) (50) (10) (50) = 17,7% of s$ $G_{7} = 927 (20) (50) (20) (50) = 2,827 of s$ (Proper = 17,000 + 5,827 = 22,827 of s (Proper = 17,000 + 5,827 = 22,827 of s	9 9 9 (A)	(152)(5.49)(22)¾ =	- 10,500	<u> </u>
$ \begin{array}{lll} & \text{Operator: } & \text{Operator: } & \text{Operator: } \\ & \text{Operator: } & \text{Operator: } & $	THE A DO STATION OF DA	M FAILING DO RE	Man(20)/5.61/c) 2 = <u>3467 . [</u>
$ \begin{array}{rcl} & & & & & & & & & & & & & & & & & & & $	FOLL THE FLUNCE	· 100000 100 112.	FILED COUNT	12.10:457
$ \begin{array}{rcl} & & & & & & & & & & & & & & & & & & & $	Charles Care	EV. 4 C.FAILURES		
$G_{F_{i}} = \frac{9/27(A)(15\%(5.67)(11.1)^{3/2}}{G_{F_{i}}} = \frac{19}{2}, \frac{7\%645}{65}$ $G_{F_{i}} = \frac{9/27(26)(5.67)(311)^{3/2}}{(311)^{3/2}} = \frac{3}{2}, \frac{827}{65}$ $G_{F_{i}} = \frac{17}{1000} + \frac{19}{2}, \frac{776}{2} = \frac{22}{2}, \frac{827}{2} = \frac{65}{2}$ $G_{F_{i}} = \frac{17}{1000} + \frac{5}{2} = \frac{22}{2}, \frac{827}{2} = \frac{65}{2}$ $G_{F_{i}} = \frac{17}{1000} + \frac{5}{2} = \frac{22}{2}, \frac{827}{2} = \frac{65}{2}$ $G_{F_{i}} = \frac{17}{1000} + \frac{17}{1000} + \frac{17}{1000} + \frac{17}{1000} + \frac{17}{1000} + \frac{17}{1000} + \frac{17}{1000} = $				
$Q_{\text{Total}_{Z}} = 17,000 + 5,827 = 22,827. cfs$ $Exercise = 17,000 + 5,827 = 22,827. cfs$	GF = 5/27 (A	Xiss Care XXIII) 3/2	= 17,7/642	
$Q_{\text{Total}_{Z}} = 17,000 + 5,827 = 22,827. cfs$ $Exercise = 17,000 + 5,827 = 22,827. cfs$	Gr. = 8/27 (20)(5.67)(311)	5,827 cfs	-C.
POU EMPORT EXCENSIVE FRANCE OUDER 100 NO NO FRANCE CONDESS.	Strong Fig. 1. West	-	and the second s	
	070+2 = 17,000	0 + 5,827 = 22,8	37 cts	
	TOU EPOST THEOUT	HAN FAILURE UND	10. 100 va FD.	ond coopyrights:

STORY = GRIVER + QFAILURE

$$Q_{1} = \frac{9}{100} \frac{100}{100} \frac{100}{100} = \frac{9}{100} = \frac{9}{100$$

SECTION NUMBER	CHAMMEL BIN E			MIN EL GRUDNO	DISCHARGE	CWSEL	CRIES	E 6	TCAL,U	104-5	TIME	AOF
312.2		ວ. ນ້	- J.S		1 10540.63	522.74	U. 0	523.54	199.40	21.26	0.03	28.29
312.2	3 300.63	J.J	ن . ز	519.2	3 31.03.63	532.22	٠.٠	533.12	404.04	V.12	J. 6 3	14.60
312.2	3 303,00	0.3	ن. د	514-2		532.43	U. E	533-76	444.44	9.15	0.63	78.17
312.2	3 300.00	3.3	٥.٥	519.4	2 4#100.00	557.17	0.0	530.13	444	7.39	U. 33	102-41
352.3		0.0	٠.٠		10540.03	523.40	0.4	524.40	197.45	21.13	0.05	44.00
352.3		J. J	J. 0		3146.0	312.32	0.0	513.54	0 L 8 . U.Z	13.95	0.64	110.00
352.3		د . د	٠.٠		3 110	511.13	٠.،	334.19		10-37	0.04	124.64
352.3	463.66	4.3	J.0	514.4	3 40360.03	517.45	0.0	514.45	414.45	4.26	9.04	474.45
362.1		3.0	3.0		3 16540.00	524.40	Ç.u	525.23	189-70	17.45	0.61	50. 30
362.3	19 430.63	3.3	J.0		3 31.06.63	532.14	0.0	224-13	503.34	13.00	0.46	10-25
362. 3		3.3	J.J		3 134603	553.32	٠.٠	>34.74	516. ,>	13.31	ودين	170.21
342.3	4 437.00	2.0	J.0	515.4	3 46100.33	537.48	9.0	538.95	541.11	17.48	0.45	244.45
372.4		0.0	0.0		9 16540.00	524.98	0.0	5.5.70	146.60	13.91	0.38	66.97
372.4		4. 3	0.0		3 116.0.10	513.44	3. 0	534.54	440.34	13.00	0-04	160.20
312.4		U. J	٥. ٥		د درون بدو و	533.41	G. C	313-44	****	13.30	0.66	1+8.12
372.4	4 300.00	2.3	٠.٠	\$13.0	6 481.0.03	511.66	J. U	214.72	544.13	11-44	J. 04	210.63
382.5		U. J	J.0		3 10546.03	525.47	0.3	524-44	164.00	15.22	0.10	84.58
382.5	5.3.43	3.3	J.0	914.4	1 31000.00	531.04	₩. ₩	537.24	5 + 4	13.75	U. U.	510.44
387.5	5.0.03	0.3	0.0		i) 1600.00	\$34.50	0.4	534. **	0 UV. 3 0	11.18	0.08	244.54
362.5	500.60	0.3	3.0	514.2	0 40166.63	514.47	٥.٠	534. 15	474. VY	10.40	0.60	361.45
397.4		v.0	١.٥		3 40543.50	5.4.74	U . U	5/1.50	144.44	15.34	0.13	104.05
392.6	6560	۵.۵	0.4	916.0	31cuc.50	515.08	0.0	310.60	4 3 L. U.	v.53	0.10	274.11
342.4	15 64.,65	٠.٠	2.3		3 33600.00	535.73	0.0	330.61	631.34	9.42	0.13	322.72
352.4	450-06	٠.٥	0.4	916.3	W 40700.L3	239.61	٥. ٥	343.56	7,5.06	7.78	0.10	474.43
\$9.1		111.21	511.Cu		4 10540.00	324.74	0.0	327-12	240-24	13-31	0.13	105.43
37.4		531.21	533.00		3 31-13	534.14	•.0	220.04	#11.AF	50.42	3.10	200.15
37.1		531.21	513.00		3 1150,.00	535-61	0.0	34.12	652.41	13.14	~~ 10	ag Land
35.1	10 10.03	531.21	511.00	310-6	44/00400	339.11	u, u	546-54	153.02	38.37	3.10	110.00
39. 2		531.21	\$11.00		10540.00	526.78	0.4	527.41	194.50	13.25	4.13	luw.ai
39.2		531.24	511.00		.U 110,0.C3	534.05	o.)	534-13	e20.14	44.47	0.13	304-04
١٠٠٧ز		531.24	531.43		U 3561 U	535.12	4.4	556- 40	637-12	14.11	0.10	172.46
31. /	10 10.00	531.21	\$33.CJ	516.5	3 4#100,03	314.61	ŭ.u	540.62	133,00	37.44	0.15	48U. 33
39.		0.0	0.0		J 1654U.00	526.74	4.3	527.41	144.34	13.24	0.13	100.44
34,		0.0	3.3		is straying	535.31	0	334. 14	417.44	0.74	0.10	161.11
39.		3.3	J.0		11604603	23 AT	4.0	534.42		0.44	0.13	3.1.42
39.	PO 1.00	0.3	0.0	516.0	3 48700.00	534.12	Ç.u	540.65	113.04	1.60	9.10	440.44
402.		3.0	J.J		10.10546200	527-13	0.0	527.81	171-24	15.76	0.14	121.85
432.		3.0	J.J		10 11000.00	513.21	٠.٠	534.46		13.86	0.12	404.43
402-		0.0	0-0		20 13600,00	535.86	4.3	53/.3.	631.20	12. 13	9.11	365,31
402.	23 340.CO	0.0	0.0	517.2	34 44100.00	334.41	U.G	Salesia	*61.13	10-61	0.11	544.34

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SECTION	CHANNEL PIN E	L OF THAK	EL OF	#14 EL	DESCHARUE	CoSEL	CAINS	EG	TOPHIU	104.5	TIME	VOL
	LENGTH SCACE			GRUUNG	ECFSC							
412.5	3 510,6-	J.3	٠. ن	544.40	13540.00	527.92	٠. ٠	524.71	140-24	13.47	7-14	139.50
412.6	3 580.00	J. u	فاندف	914.46	31500200	516.30	٥.٠	537.49	514. 04	7.61		J46.16
412.6	3 510.00	0.3	ں ۔ ں		1100000	516.VJ	u . J	551.92	5/1./0	4.17	5.13	421.00
412.4	3 510.60	3.3	3.0	514.43	4.100.00	540448	9.4	341.60	414.46	9-20	7.17	414.41
422.9	£ 405.C3	6.0	J.0		10540.00	520.62	0.0	529.17	261.30	8.50	0.16	155-65
422.4	1 405.00	0.0	٥.٥	516.13		3,0.81	٠.٠	537.44	5/4-44	1.44	0.15	441.64
422,4	1 405.00	0.0	J.G		. 31540.Cu	937.41	ن ۽ ن	534.24		7.0L	3.12	4404
422.4	1 405.60	٠.٠	3.0	510.10	4.1060	543.41	ù.u	541.45	611.30	1.45	3-15	736-88
432.*	9 425,60	2.0	2.0	514.84	10540-00	524.44	0.6	527.62	322.44	12.05	J. 20	174.19
432. 7	4 425.43	٥.٥	J. C	510.00	31	531.04	٠.٠	510.04	434.40	4.47	3.11	3-6-15
432.4	M 452-CD	٠.٦	.	510.44	110-0-43	517.65	Ģ.u	534.69	1:5.30	4. 41	3.10	330.53
432.9	¥ 425.30	٠. ٠	J.J	364.6	441.0.00	341.26	4.0	342. 11	186.04	8. 45	0.10	108.25
443.0	433.60	0.0	0.0	\$15.40	16140.30	524.34	Ç	330.40	254.34	10-16	9.22	147.33
443. 0	1 413.03	0.0	0.0	515.40	11600.03	531.31	u . J	554.49	144.17	4.47	3.14	591.10
443.0	3 430,CC	0.3	٠. ن	517.00	, 310 cu. 0a	>>1.79	0.0	331.14	101.02	9. 29	0.15	372.14
443.0	430.00	c.0	0.0	515-4	44/40.00	341-41	۵. ۰	542.20	* 44.34	8. 16	U-15	612.33
453.0	130.00	0.0	3.0	517.0	105403	524.70	0.3	530.24	205. **	0.37	9.22	144.17
653.0	29 130.00	0. J	0.0	51/	1 11	537.22	9.0	534.11	224.47	12.73	J. 18	364.47
453.0	133.53	٥.٥	٥. د	517.3.	3 16 3	531.15	0.0	514.14	224.00	11.42	7.10	c-0.31
453.6	9 130.00	٥.٠	0.0	\$11.0	4410	940.43	0.4	34 3 . 14	330-BA	14.33	0.14	491.50
45.1	0 5.00	514.40	535.20	517.0	3 10940.00	525.55	0.0	510.29	164./-	20.44	0.22	144.34
45.1	نه و ا	514.83	535.23	517.0	. 316	536.57	٥.٥	518.64	224-44	41.35	3.14	544.44
43.4	0 1.00	538.83	213.23	547.0.		534.70	0.0	218-07	226-05	114.01	3.15	0.00-15
43.1	S	118.80	>35.20	517.0	44140.03	534.09	0.0	5-3.00	224.00	234.52	0.16	444.31
45.2	25.00	510.64	\$35.20	111.6	1 10540.03	5/4.61	0.0	330.34	144.77	20.25	0.22	444.34
45-2	20 25.00	518.80	535.20	311.0		>30.01	٠.٥	> > 14 - 13	220.00	81.05	4- 14	566.40
45.4	25.00	933.00	333.40	511.0		531.10	0. 0	214.46	224.45	110.01	0.10	60 d . 35
45.4	10 25.00	530.43	335.20	547.0	44103-63	534.44	U. u	544.40	220.01	234.52	3-10	PV 1-91
45.1	1.00	0.0	3.0	517.6	0 10940.60	524.61	0.0	430. 14	184.77	20.24	0.22	144.43
45.	1.00	٠.٠	J.0	511.0	J. Steeling Co.	531.14	0.0	514.24	220.50	25.46	0.14	36 A. 53
43.	50 \$.CC	J.0	0.0	911.0	1 11800.00	510.42	6.0	54 0 . 0.2	i20.aa	26.54	J. 18	
45.	1-03	0.0	0.0	211.6		942.69	4.3	544-11	220.71	20.23	0.10	***- 00
441.	14 235.60	0.0	u. 0	514.1	3 10540.00	530.04	3.0	510.14	204.14	14.47	0.25	204.91
443.	14 211.00	3.0	0.3	516-1	3 1:000.00	514.00	ن د ن	339.12	545.15	14.01	0.20	547.72
453.	14 235.00	0.3	3.3	510-1	L discussion	537.69	u. .0	540.51	518.70	13.44	6-14	632.54
461-	14 215.00	3.3	٥.٥	516.4	44100.00	344.54	3.0	543.24	461.33	9.47	0.14	A5A* 13
413	25 551.00	2.0	J.0	517.5	4 10540.00	930.41	0.6	514.56	173.44	11.77	3.20	414.54
473.	25 555.CJ	J. U	J.0	211-5	£ 11000.63	534.27	.	94 Lu 6 J	442.00	14.45	4.71	651.66
4/3.	25 555.CG	0.0	J.0		G 33000.0C	500.04	٠.٠	341.40	4>5.44	14.57	0-31	130.43
473-	25 555.GO	0.0	0.0	31/-5	J 48100.CO	544.44	0.0	343.78	559.46	14.11	0.21	1010.51

	MARREL PER Ength #340		EL DF CHURD	MIN EL DISCHARGE	CHSEL	CRIAS	EG	TOPELU	10x+S	TIME	AOF
443.29	233.00	3.6	J.3	211-02 10240-02	530.86	6.0	531.97	131.54	16.70	0.27	241-37
483.29	230.03	3.3	J. J	517.03 11006.03	534-12	u. u	341.47	140.44	24.55	3.44	664.73
441-29	233.65	J.J	J.0	517.00 33600.00	536.66	c.,	517.34	171.00	31.35	3.21	119.82
48 1. 29	230.00	J. J	3.0	511.43 4#1.4.13	544.43	334.14	547.10	+13.1/	34.57	3.21	1054-51
48.10	5.40	539.40	511.50	517.00 10540.03	530.67	0.0	531.ve	131.57	16.67	u. 21	241.53
48.10	5.00	939.40	515.53	Silvo dicerco	531.48	ن ـ ن	541- 60	101.07	140.04	0.22	867.74
48.10	3-60	539.40	>15.50	511 310cc.c3	5 14 . 74	J.J	542.6>	141.49	105.40	الدون	1.0.04
44.10	5.03	534.40	535.50	511.05 48103.03	>>>= 1	:34.44	544.16	1+1.40	147.59	0.21	1058.89
46.20	25.00	319.43	515.50	517.00 10540.00	530.92	4.0	512.02	131.77	14.44	0.21	242.31
46.23	29.60	934.40	313.30	Silena Biacoral	537.03	3.0	24 / 21	142.34	140.44	0.22	6/1.12
48-23	25.00	337-44	515.50		537-10	٠. ٥	553.24	145.24	145.44	u. 21	121.22
48.20	25.CU	539.43	515.50	311-63 40100.63	514.84	514.44	544.63	441-07	347.59	0.21	1000.41
44.25	1.00	0.3	0.0	\$17.00 12540.00	550.45	0.3	512-02	1)1.71	16.45	0.27	242.41
41.25	1.63	0.3	0.0	511-03 31603.63	534.16	c. o	347.44	144.60	22.42	0.22	6/1-16
44.25	1.03	0.5	٥.٥	341.63 13600.03	340.73	د. د	241.29	LvI.le	22.14	4.26	721.20
44.25	1.60	0.0	0.0	517.0¢ 407.0c.uJ	347.44	534.16	550.14	411-65	10.62	v i	1949-12
44.30	623.30	0.0	0.0	517.60 16540.33	532.01	0.0	512.52	112.45	12	3.24	264.26
40.30	#:U=00	J.J	J.J	517-00 31000.00	341.54	ن . ن	543.30	378	16.42	4.43	743.42
44.33	620.00	J. J	0.0	\$17.03 \$34003	312.44	ں ۔ ن	>+4.61	415.00	45.72	٠.23	fac.44
44.30	£23.#3	J. 3	0.0	511.00 44100.00	544.08	0	351-29	463.40	4.46	0.23	1155. 61
493.42	10.00	0.3	J. 9	521-43 10546-60	532.47	532.67	535.12	102.20	176.76	0.29	264.55
441.42	10.00	٠.٠	y.3	351.44 \$100000	345.11	4.4	541.78	201.03	41.74	4.25	714.54
49:42	13.63	٠,٠	4.0	251.00 31600.00	343-24	٠.٠	500.45	3,4.34	35.02	0.23	769.46
443442	10.00	٥.5	0.3	528.43 4016G.CJ	550.14	J.0	331.44	425.25	10.01	0.21	2157.41
50.35	540 cm	0.0	3.0	925.10 10540.00	536.33	w. C	536.84	257.50	11.24	0.12	203.05
50.15 50.15	540,03 540.03	0.0	٠.٠	525.10 1100000	541.13	4	>44.78	124-04	12.47	0.25	131.21
50.35	540.00	0.3	٠.,	521-10 310-0-0	344.41	3.0	343.94	171.46	12-74	9.25	010.44
,,,,	70.00	0.3	0.0	525.10 48830.00	551.12	3.3	552.45	4/6.24	4.13	4.23	1239.46
\$13.45	650.00	3.3	3.0	525.63 £0540.03	511.22	ن , ن	557.47	235.44	14.75	4.15	312.55
\$11.45	643.30	4.3	١.،١	525-80 316-0-03	344.04	٥.٥	346- 18	414.uL	14.43	4-21	424-44
343.41	443.63	0.5	٠.١	525-84 33426-13	343.18	J. U	291.05	208020	10.01	w.21	484-43
511.45	48 0.00	٥.٥	0.0	529.80 48703.00	551.58	0.0	552.41	334.23	17.18	0.40	1340.44
521.40	140.00	0.3	0.0	\$22.80 10540.20	510.24	J. 0	534.74	114.40	4.49	J. 19	349-74
523.43	193.60	٠.٠	0.3		544.23	د. ب	344.77	4/1-19	4.49	J. 51	Ang. 25
523.43	193.63	3.4	0.0	\$22-60 \$1600.03	34/.14	٠.٠	54 / . 44	4/9.24	4.69	د ه د د	413.65
523.60	149.03	3.3	3.3	522.43 44760.63	552.25	4.0	553.42	315.14	4.45	0.31	4411.30
111.42	660.03	ə.a	3.0	\$24-23 80140.00	334.42	0.0	534.42	174.70	13.12	4.42	383.74
\$33. 12	600.03	0.3	J. 0	\$24.20 \$1000,00	540.10	9.0	547.41	4/0.00	10.52	0.33	477.84
533. 52	860.01	u.J	J.J	329.23 31633.03	541.45	J.J	544.83	Sileva	11.45	3.32	1-50-32
\$ 11- 02	460	4.4	0.0	Secret Addances	443.41	2 - 0	554.02	444-41	10.04	0.55	1134

SECTION NUMBER	CHARMEL MIN		EL OF	MIN EL	DISCHARGE	CWSEL	CAINS	EG	TOPULU	tor+\$	TIME	AOF
53.1	0 540.00	٠,٠	ن د پ	525.00	14546.64	534.45	u.q	539.94	142.44	6.48	0-44	402.66
53.4		0.3	ن. ن	525.00	3 316-0.63	541.42	٠.٠	340.10	449. +1	10.63	U-34	1022.93
51.1		0.3	0.0	523.0	1 16.0.JJ	548 29	0.0	544.54	499.00	10.11	0.34	1136.62
53.1		4.4	3.3	525.00	44400.03	553.40	4.4	554.54	دن جنال و	1.14	0.45	1684.52
53.2	3 5.00	544,83	543.20	525.C	10543.03	539.44	٥.٥	537.96	142.70	4.97	U. 44	442.67
\$1.2	0 5.00	544.44	540.20	323.0.	. 31:30-33	547.11	ں ہاں	540.00	499	41.74	0.14	1023.30
\$3.2	5.00	544.63	543.20	525.4		548-16	J	549.64	475.70	45.18	J. 34	11-0-53
53.2	3 5.00	544.40	543.23	525. C	44100-03	553.44	3	354.54	3,44,43	21.14	U.35	160 71
51.1	19.00	544.63	544.20	\$25.0	3 13500.00	339.47	4.0	539.48	142.44	4.45	J.44	443.52
53.5		500.03	300.20	523.0	• 31-303	547.34	3.3	544.53	449. 26	73.41	0.34	1024.53
51.1	13 15.00	544.43	>+>.20	525.0	الأبارة والملاوا وال	340.35	J.J	569.73	9 4 7 . 7 7	.1.34	4.34	1171-84
53.3		544.66	340.26	525.0	483353	553.71	J. 6	550.59	320.00	24.##	0.35	1941-63
344.3	1.00	0.3	٥. ن	525.0	: 13560.60	539.47	J.u	534.97	142. **	4.95	3.44	443.57
544	1.00	J. J	J. 0	525.0	3 31303.03	541.14	ن. ن	344.46	479.7/	Y . #4		1-24.62
544.	1.00	0.5	ن . ن	525.0	2 13630.00	\$46.45	0.0	544.12	416. **	9.41	U. 34	11-7.50
344.	1-00	0.0	٥.,	52>.6	4 4 4 4 5 4 5 4 5 4 5 4 5	\$53.54	0.0	>54.03	546.43	7.55	0.12	1015-14
54.1	100.00	J.4	0.0	524.5	J #J940.22	111.19	0.0	340.09	143.50	4. 46	J. 45	427.66
54.1	6 100.00	3.3	ن د د	324.5	3 31003.03	347.34	0.6	54 1-22	313.30	14-15		1012-00
54.1	10 103.60	3.7	J.0	524.5	3 31603	540.02	J.J	54 4. 94	343.34	14-65	J. 34	1117-44
54.1	103.66	3.0	٥.٥	524.5	\$ 44160.CO	\$52.44	٠.٠	\$54.92	341.4>	12-12	0.35	1726-41
54.2	10 5.00	544.00	514.43		3 13546.30	534.39	4.0	54 J. US	140.55	4.90	0.45	4.1.46
54.	10 9.66	544.00	5 34 . 43	524.5	j 11000.03	945.16	534.43	544.03	243.61	141.03	J.35	1011.15
54.	20 5.00	544. 00	534.43	524.5	J \$ 3600.4J	540. 14	J. U	550.54	Jul. **	101.56	U. 54	1111-14
94.	20 5.00	544.03	314.40	524.5	0 44105*30	392.40	9.0	554.94	341.70	44.13	u. 36	11-4-91
54.	25.00	\$46.00	5 39 - 43	524.5	C 10546.LO	5,9,43	0.0	540.13	140.14	22.34	U.45	434.83
51.	10 25.JC	444.20	311.44		\$ 31CUC3	541.15	0.4	350.44	340.00	1.6.32		1012-14
54.	36 25.00	544.00	514.90	524.5	J 11600.00	948.30	4.3	550. /e	3:0.4:	176.15	J- 14	1114-11
54.	30 25.60	544.30	\$37.43	524.5	£ 48/00,03	553.22	9.0	555.14	341.22		U.32	1104.42
\$54.	35 1.00	0.0	J.J		3 10540,03	519.43	0.0	540.15	140.14	8. 54	J. 45	4-1.67
554.	es L.00	0.0	0.3	524.5	2 31000.03	544.12	0.4	554.42	118.50	10.42	J. 35	1035.24
394.	4.00	0.3	3.0	324.5	J 33000.	541.24	0.0	35J. 84	142.74	11-12	0.34	1115-41
554.	05 1.60	0. 3	0.3	524.5	3 48366.63	553-13	J.3	355.12	141.94	12.11	V. 36	1174.14
544.	9 211.03	0.0	0.0		3 10940.30	539.47	0.3	540.15	204.50	9.91	J. 48	417.63
344.	311.00	J. J	0.0	524.8	3 31600.00	344.86	9.0	293.21	384.14	8-38	0.34	1057-18
544.	04 211.03	3.3	J.4	5.4.4	J Jacque J	554.45	y. 2	551-14	344.30	4. * 3	2.15	1146.46
564.	04 211-03	4.5	0.0	524.8	3 44103.33	554.54	4.0	555.44	146.03	4.51	0.36	1110.00
514.	19 525.60	3.0	0.0	523.4	C 44540,00	544.36	0.0	540.84	345.70	0.47	J.49	444.03
574.	19 525.00	0.0	J.0	543.4	13 11Ccc. 33	550.21	v. J	550.80	110.31	6.26	0.14	4144-44
574.	19 525.60	ں ۔ ں	0.0	321.0	3 33600.00	550.47	0.3	551.34	331.36	4.59	J. 14	1210.54

	CHANNEL PIN E Lenuth Frace			MIN EL CACUNG	DESCHARGE	CMSEL	CRIOS	EG	TOPALL	10K+2	TIME	ACF
584.27	423.30	J. J	ن دن		1 10500.00	540.49	0.0	541.12	3/1.44	4.34	0.52	410.50
584.27	423.00	3.3	د. س	323-6	C 3100000	550.12	j	551.10	367.14	3.15	J. 44	1101.51
544.21	420.00	J.J	0:0	323.6	3 33000.03	551.16	0.0	351. 11	346.00	Ja va	3.45	1213.60
544.27	420.60	4.4	U. U	525-6	3 4076J.CJ	555.55	0.4	556.12	444-10	4.34	0.43	1443-43
554.36	413.63	ə. ə	0.0		6 10546.03	561.13	0.0	541.50	837.24	1.34	w.56	510-40
594.36	470.00	٥.٥	0. ∴		i 31.00.00	551-15	u.ŭ	551.20	1445-12	u. 16	0.47	1315.57
514.36	410	J. J	J.0		j 316.v.LJ	551.02	0.0	552.00	1504.44	U- /4	U.47	1412.50
554.36	412.3C	9.3	3.0	524.6	44/00.00	>>4.18	٠.٠	354.23	1004-07	4.64	0.47	4084.44
4.55	1020.00	0.0	0.0		4.540.60	540.81	0.0	542.25	134.14	20.68	J.50	5/2.04
4.35	1070.00	0.3	U. J		. 11000.00	551.44	لاءن	951.20	1511-10	0.57	0.4.6	10/4.04
4.35	1020-60	0.0	3.0		i lynusia	551.71	J.U	351. 45	1514.26	0.04	. 60	1/10.45
4.15	1020.00	6.0	٥. د	524.1	C 3+100-CJ	556.25	G. J	336.2V	1152.04	4. 56	V.34	1010.11
4.55	1.00	544.41	514.40		£ 1054C.Co	540.50	u.a	542.13	101.40	12.15	0.59	5/2.06
4,55	1.06	544.41	344.40		u direction	101.25	4.3	551.24	1511.14	6.14	U	16/6.78
4.55	1.00	544.41	511.40		U 15000000	511.42	ن . ن	351.45	1514.94	U. 59	J. 66	1141-18
4.55	1.00	544.41	714-40	520.7	6 34102.34	550.25		224.SA	1752.04	U. 15	9.64	40-01
614.55		544.41	517.40	5/2.7	J 16940003	540.64	٠.٠	5+2.41	132.52	72.12	4.59	572.53
614.55	10.00	544.41	\$39.40		4/202463	551.25	J. J	551	1511-04	J. 66	J. 68	1047.01
614.55		544.41	244.47		C Extrema	551.42	0.0	551-95	1514.90	U. 49	9.66	18/1.49
414.55	20.00	544.41	334.40	520.7	. 30700004	>>6.45	0.3	228.38	1152.01	0.75	0 -4 6	3696.48
4.55		0.0	3.0		0 10540.00	141-14		542.53	101.31	18.40	J. 59	5/2.54
4.55		U.J	٠. ن		i Hunany	551-25		351-40	1511.44	0.57	J.68	1073.14
4,55		J. U	0.0		v 196.0000	221.41	J: 0	351.45	Ibi te ua	0.61	U. 0 .	1003.01
4.55	1.00	5.0	0.0	523.1	0 34577-00	554.25	0.0	110.30	1152.43	U.50	U- P9	1646.56
624.73		0.0	4.3		J 13540.00	542.65	0.0	342.94	1421.20	0. 90	2.15	450.70
424.73	450.00	4.0	0.0		u libraria	151.24	J. J	>>1.31	1463.04	o.la	0.44	2011.04
624.73		9.3	0.0		0 14200-00	311.41	ن. ن	221.48	1406.10	V+21	J. 4J	2103.34
424.73	430.00	٥.٠	u. 0	341.4	6 14/00.00	>>0.30	٠.٥	356.33	1440.41	0.21	0.62	3141.94
434.42		0.0	0.0		3 13540.00	542.91	٥.٥	342.96	1215.21	1.44	3.61	729.43
# 3 4 . # Z		0.0	J. 0		3 1/00/200	351-36	4.6	35 6 0 32	1080	4.20	1.00	2212.31
434.42		J.J	٥.٥		0 14602.03	351.48	٠.٠	552.00	1111.04	0.23	1.01	2360.48
634. #2	416.CG	٥.٥	0.0	501.4	u 14100.00	554.32		354.33	1111.10	3-21	0.42	3410.19
4.42		0.3	0.0	534.4	J 1654J.BS	542.79	542.19	\$40.56	41.74	117.77	3.44	172.54
4.12		3.3	u.J		2 11000.00	551.11	J_J	351.43	1200.00	5.44	1.14	2 15 84 45
4.92		J.J	0.3		3 19500.03	551.02		352.44	1212.06	3.53	1.07	2524-14
4.92	\$43.60	U. U	3.0	534.4	3 34300.00	356.21	J.J	550.W	4144.33	2.50	0.48	1005.47
4. 42		354.50	544.10		£ 10540.00	541.33	ن . ن	340.61	42.04	43.42	U. # 4	112.60
4.42		524.53	344.10		3 1000.00	551.23	 .	351.41	1214-02	4	1.14	4354.17
4.92	1.03	554.50	344.10	334,4	7 14017-00	331.YI	ن ـ ن	352.04	44.00.00	4.4/	1.07	2524.92

BER LENG				54 X14C	BCFSC								
644.93	.TH RCACHA 35.03	554.50	549.14	534.40	10540.00	544.40	0.3	544.95	45.01	42.24	0-44	773.23	
44.93	35.00	550.50	344.10	539.45	11000.00	331.25	٠. ١		1218-00	9.51	1.15	2302.30	
44.93	35.00	554.50	544.10	554.44	19656.00	551.45	0.0	552.62	1100-11	8.75	1.08	65 cv. 13	
44.93		354.50	544.10	514-44	34100.50	550.31	0.0	556.42	2120.06	3-60	4.84	7647-77	
4.13	1.00	4.0	3.0	534.40	16546.63	544.43	6.0	544.94	46.41	41.25	0.84	223.25	
4. 53	1.60	0.0	0.0	514.40	17600.00	551.40	3.3	551.48	1209.02	5.82	1.12	. 342.48	
4, 93	1.00	4.0	7.5		14400.03	351.64	0.4		1334.49	5.34	1.47	25.4.49	
4. 73	1.00	ŭ. 0	0.0		34100.00	554.2V	0. 4		414040	2.47		3443.62	
			3.0		16546.03	540.60	0.0	544.60	646.74	37.26	4.86	N1.05	
	526.40	0.0				551.44			1104.10	11.45		.4/3.73	
	524.66	2.3	0.0		1 four ca		4.4						
	52 6. Cú	3.3	7-7		12600.00	552.15	4.0		11531	16.44		2001.00	
455.03	528- 66	3.3	نے ق	314-40	34700254	\$50.30	4.4	276.68	1240.39	1.20	1.02	3441.47	
645.19	445.00	3. 0	0.0	541.00	10540.00	552.81	0.4	554.62	e21.33	100-24	₩.##	472-34	
885.19	443.60	0.0	0.0	541.46	110.0.00	554.14	354.14	550.48	244.24	111.35		2-04-42	
445.19	845.66	0.0	J. 0	547.00	1400-00	554.42	3544#1	557.24	342.41	112-68		201-27	
445.19	445-46	0.0	2.0		34166-00	554.5U	558.50	561.26	144.74	44-94	1.00	7414-22	
				4	1.546.00	95+. Ju	559.33	562.62	151.3>	46-37	0.40	451.31	
475.35	845.00	3.0									1.14		
675-35	445-44		٠.٠		1100.00	304.20	562-49	564.55	140,30	41.11		2511-24	
415.35	845.64	٠,٠	4.0		19400.63	363.24	543.24	>0/4 44	440.45	41.14		2101.50	
413.35	045.63	0.0	0.0	344.70	34100,00	\$44.75	:44.33	5/3.04	420.00	34.44	1.03	101.00	
485.51	845.60	3. J	0.0		10140.43	561.13	4.0	564.54	144.10	31.79	3.93	861.44	
445.54	845.00	J. J	0.0	350.00	11,00.00	310.01	4.0	511.77	211.11	44.53	1-13	2541-71	
685.54	845.60	3.3	3.3	355-66	190.00.00	511.45	0.0	5/2.64	211.14	42.17	1.10	2715.21	
445.51	845.63	3.3	3.0	3544.00	\$4700.63	514.66	3.0	571.52	201.47	46.14	1-01	4498-35	
ECT134	DISCHARL	E Cale	Ł	Casel DIFF	. CASEL C	LFF CHSI	EL-BSELA	TOPULD	T.w. 01	FF LENG	.TH		
MARER	CHS		•	ERCH Q	EACH SE								
\$12.050	10540.60	3 51	3.237	4.0	0.6		9.0	130,424	0.0		0.0		
212.050	31660.66	. 51	4.131	4.476	U. L		0.0	140.424	~#. 9		3.4		
212.450	\$3600.60	O 52	J. 523	4.614	٥. د		0.0	174.345	-21.41) - J		
212.050	4#742.66		3.222	1.411	s. c		0.0	301.313	-424-50	16 (3.3		
222.010	10100.00	3 41	4.324	٥.٥	3,6	47	0.0	165.491	w. 0	4.44	د ده ده		
222.073	31103.00		5. 284	8.94C	5.5		0.0	144.636			3.304		
				3.531	5.6		0.0	150.450			4.433		
333.0 M	1160.00		4.145	4.213	5.1		0.0	200.417			0.000		
-													
212.040	10540.00		P- 520	0.0	-0.0		0.0	197.494			7.100		
\$33.040	11 000.40		4.404	4.711			0.0	114-143			9.000		
212.040	\$34	. 3 32		3.411			0.0	142-972			0.00		
\$33.440	48144.60	i) 51	10.454	4.343	-0.2	•	9.C	422.493	-200.5	44 10	0-64		
142.100	£0540.CC	ı q 51	5.145	3.0	-3.4	66	0.0	150.762	J_ 3	•	ودد. ه		
242.100	31.00.00			4.475	-0.9	04	0.0	114.670	-14-9		0-633		
242.100	11600.00		13. 124	0.464			0.6	111.604			3-044		
242.100	40100.66		4 . 110	3. 941			3-0	192.174			0.000		
4	10441 5			3.3	1.1	۸	0.6	649.248	U. 0	1	c.000		
24.100	10140.00						0.0	373.007			0.000		
24.100	11630.00		24.254	12-144									
34-100	33463.00		14.359	3.416			0.0	314.012			3.300		
34.100	40106.61)) 5 <u>5</u>	11.176	2-613	2.4	44	0.0	492.200	-302.1	>4 f	0.000		
252.110	10540.00	3 51	16,723	4.0	-0.1	13	0.0	144.814			0.000		
			24.454	12.140			0.0	364.464	-217.5	91 1	سرر ر		
242.110	31 666 6	4 ايان											
292.110	31666.6		/4.1d4	3.343			0.0	374.137					

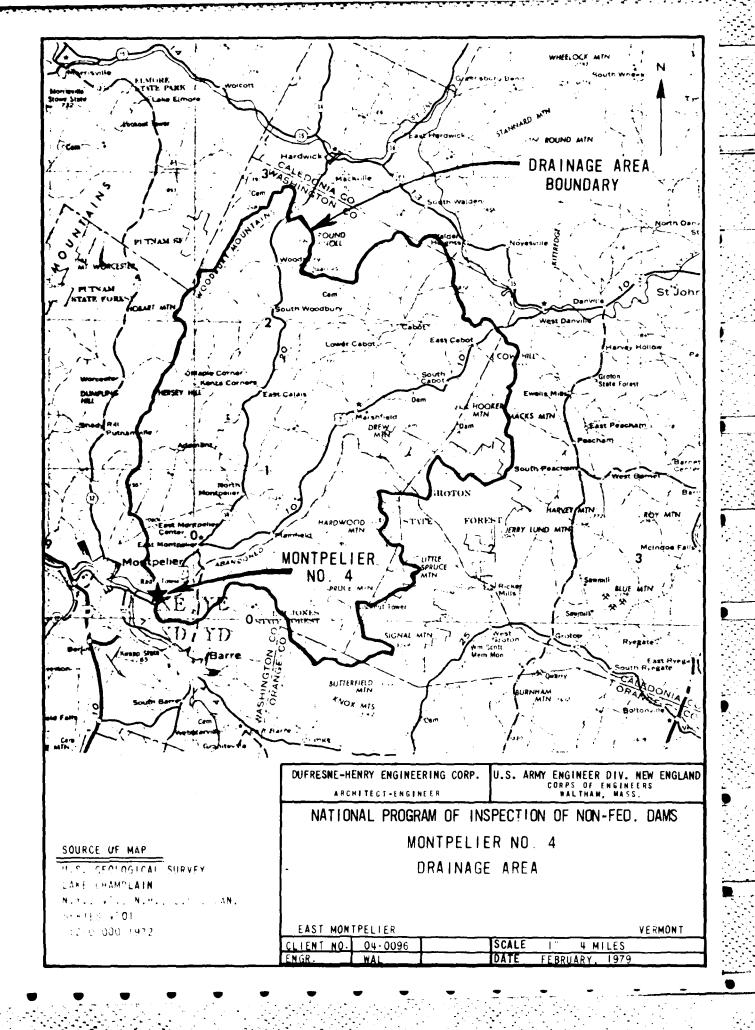
262.120	10540.000	514.704	0.0	1.989	0.0	132.256	٥ ـ ن	100.000
262.123	31003.003	521.130	4.842	-1.404	0.4	441.950	-1.644	400.000
262.123	316.3.363	521.718	U. C14	-1.470	0.6	101.450		100-000
262.123	46106.663	333.341	2.193	0.650	0.4	340.433	-213.811	100.000
20.330	10:43.003	514.231	1.0	0.523	c.c	133.422	J. 0	41.000
20.333	\$100000	524.433	4.749	4.200	0	18v. 25e	-52, 034	41.000
20.300	33600.000	>23.440	-3.034	1.220	0.0	141.3.3	-52.004	41.000
26.330	44100.600	533.6/1	4. 325	0.160	C.4	350.174	-223.151	41.000
272.150								
	13543.603	251-340	0.0	2.155	0.0	161.504	٠.٥	190,000
412.150	Michaelia	231.013	6.424	5.030	0.0	241.034	-115-431	130.333
272.153	31600.000	\$11.472	0.442	2.524	u . u	305.513	-624.110	133.000
272.150	48703.600	535.540	4.074	4.473	c.c	341.945	-154.101	133.000
282.470	10546.600	921.771	3.4	0. 384	0.0	193-441	0.3	100.000
242.170	31603.	531.644	4.414	C. 0 / 4	U. 9	177.635	-200.375	130.000
242-173	33 * 3 3	532.751	3.961	0.114	G. C	375.451	-200.344	100.000
282.113	49 146.603	530.572	4. 321	1.026	J-0	179.619	-200.438	100.000
			*****		***	******	-200.478	.00.555
312.234	13.4 1. 603	522.234	1.0	0. 167	0.0	199.445	٠.٥	103.030
315-520	31C	932.2.4	4.444	0,143	0.0	444.819	-200.154	300.000
312.233	33655.613	232-628	4.602	a. 37a	.	404.644	-28>.164	30 333
315.50	41700,000	517.1/3	4. 343	U. 594	0.0	404.463	-20>.203	300.00
352.313	10540,000	323.276	J. 0	0, 656	C . C	197.450	J	400.000
357.310	31612.600	512.547	4.426	5. 293	6.0	418.216	-42 140	100.000
352,310	11666.000	213.120	u.e.ic	0.299	U.0	616.220	-42 00 110	460.000
354.313	4870	53/.422	4.329	0.245	0.0			
	*************	******	1.7.1	••••	0.0	814.650	-424.403	400.030
\$42.190	10941.000	524.434	J. 4	0.443	0.0	149.444	J. 0	430.000
362, 310	311.,,610	532.137	4.217	3.220	U-0	503.525	-311.624	413.000
\$45.749	31603	5:1.144	0.341	3.142	٥.٥	518.353	-32e.457	434.00
345.143	48703.603	531.401		9. 630	0.0	361.770	-311.413	430.000
372.440	13:43.603	524.943	3.3	3.521	0.6	180.545	٠.٥	300.00
372.443	110.7.003	511. 119	4.059	0.303	6.6	440.544	-251.974	306.000
312.443	\$1030.003	511.047	0.377	0.244	u - 4	444.543	-234.000	\$00.000
372-443	4470	511.000	1.495	0.126	0.6	324.735	-842.640	303.000
		,	,,,,,	-,,,,		,,,,,	-342.610	303.000
382.500	13943.600	525.613	3.0	0.664	u.c	100-065	٠.٠	500.000
312.543	11600.000	915.444	4.174	0.464	U.0	Sve. Eul	-420.710	\$00.000
342.5 W	316.0.0.3	5 54 . 4 4 6	3.652	6. 205	6.0	604.511	-441.512	300.000
302.543	4810	314.853	4.164	1.459	0.6		-548.921	\$ (). ())
342.450	12542, 500	>24.744	a.c	1.674				
342.450	116.0.633	535.3/4			5-0	144.444	v. a	450.000
344.610			4.333	1.234	9.0	630+034	~433.584	450.000
	3300000	515.726	3.64	1.559		457.314	-404-612	450.000
392.450	48300.003	>34.013	3.401	0.448	4.6	733.413	~534.364	65.00
\$4.100	10543.633	524.760	J.a	4 (10	0.0	194.512	u. 0	13.000
34.100	11603.660	514. /94	4.031	-0.247	0.0	611.412	-423-149	10.000
34.100	33503.003	\$15.012	0.4/1	-0.114	0.0	452.442	-43/.435	10.000
14.100	487.0.103	534.161	4.153	0.154	u.o	133.415	-314.303	40.000
							,,,,,,,,	
34-500	10.4).003	520.115	J.O	0.015	C.C	194.576	·. 0	16.000
34.700	316 603	3 34 - 6 34	4.043	0. 61	0.0	420.140	-4/0.104	13.3.3
39.200	310.0.00	515.722	J. 444	9.111	0.0	457. 123	- 96 5 9 9	10.000
34.200	48100.000	214.413	4.341	3.647	0.0	141.416	-514.240	4
34, 350	10546.630	525.116	a.c	0.601	٥.٥	194.5#3		
39.310	31.60.000	545.312	0.535	U. 454	0.0		J. 0	1.000
34.300	310.0.000	511.107	V.576			439.442	-442.359	1.000
34.300	48760.600	514.121		9.185	6.6	864.882	-470-299	1.600
374 300		2340144	3.414	-0,652	0.0	133.444	-534.232	1.000

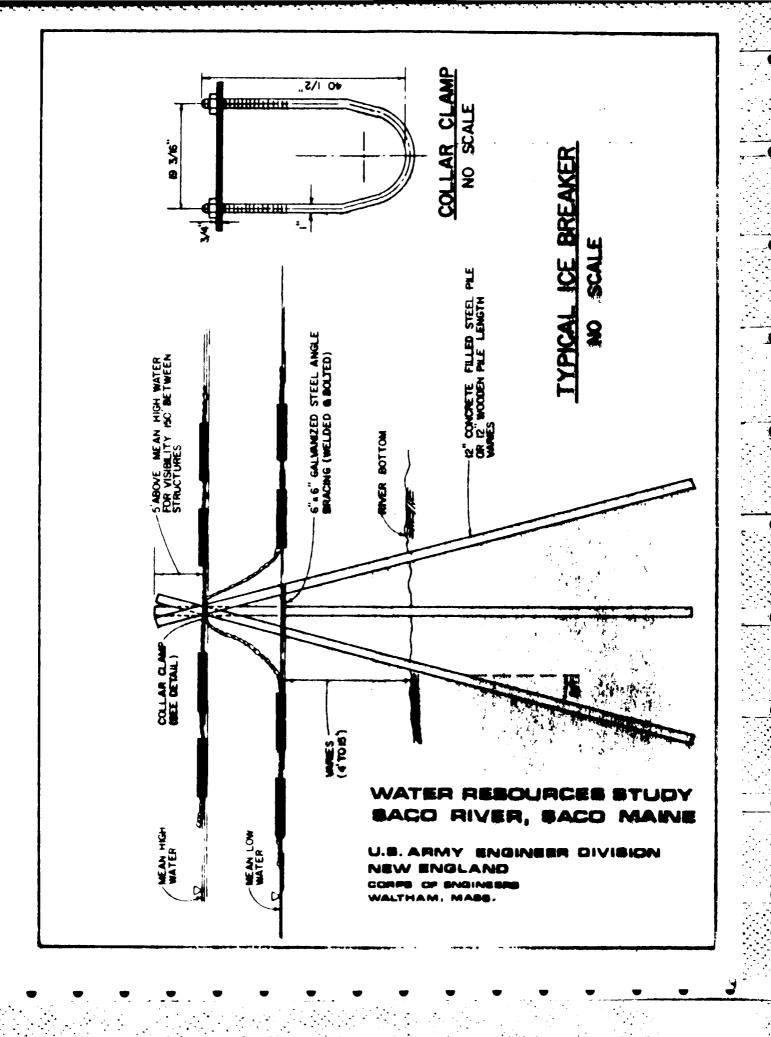
401.130	10340.000	221.121	0.0	0.333	0.6	141.231	u. 0	340.600
402.1W	31600.000	535.204	4.136	-0.044	0.0	644.544	-412-362	300-000
402.743	33666.606	535.057	J. 586	-0.654	C.C	431.700	-464.543	360.600
402.113	48763.660	539.666						
******	44700.000	,,,,,	3.416	-0.653	0.0	441-135	-443.445	340-0-0
					_			
412.830	10943.642	521.415	0.0	0. 7 84	0.0	140.923	u_0	510.000
412.630	3166603	330.214	6.361	1.624	6-6	\$14.415	-343.442	510.000
412.000	3366603	5 10 . 0 44	4.863	4.642	U.U	\$11.156	-340.833	540.000
412.030	48766.660	540.444	3.505	0-117	0.6	610.007	-433.144	340.000
*******		,					-4,,,,,,,	380.000
422.510	10140.000						_	
		524.422	0.0	0.767	0.0	261.301	v.0	•05.000
422.610	31601.600	314-413	4.144	0.514	٠.٥	574.6.99	-51	465.000
622.51 0	33666.666	53/19	3.634	0.515	0.0	444.536	-354.236	405.000
422.910	44102.000	540.411	1.556	0.484	٥.0	4/1.300	-410.030	405.CUU
					•••		*******	*******
437.590	10540.000	528.434	٥.٥	0.313	0.0			
						322.941	v. 0	425.000
432.5%	3160	53/0034	4.104	0.224	0.0	734.666	-410.473	425.033
414.440	33000.664	137.414	3.615	ن ، ي س	u.c	255.J55	-43344	425.330
432.450	4# 103. LU3	341.236	3.404	0.201	0.0	100.425	-451.433	425.003
441.073	10 543.033	529.313	٥.0	0.455	U. C	254.304	٠.٥	4 40.0.00
441.070	31000.603	53/. 167	1.971	0.321	V.0	100.110		
							-46486	بعان دو پ
443.073	31466.606	33/	0. 621	0. 140	0.0	101.010	-524.100	410.00
443.670	4.163.403	544.413	3-914	J- 333	0.8	444.524	-340-216	434
453.093	10560.00	577.0+7	ن ـ ن	3.307	0.0	249.938	U. 0	130.000
453,090	31606.600	517.45	1.521	-0.442	U. 6	420.654	-14.913	130.000
455.074	314-0.663	537.750	3.540	-0.243				
					0.0	240.437	-10.416	133.000
453.090	48160.603	540.627	2.411	-0.900	0.0	220.600	-10.421	130.000
49.100	10:46.636	574.544	J. C	-0.149	4.0	184.203	U.U	5.400
41.100	34603.603	936.572	1.021	-3.61	0.0	223.00	- 30 . 144	. 5.000
45.134	33666.6.3	536.014	3.326	-3.853	4.0	420.647	-30.144	3.000
45.100	40760.000	534.446	1.144	-2.541				
47.100	40.00.000	3,01360		-4.741	0.0	220.041	-10-174	5.000
					_			
43.200	10940.050	521.611	J. U	0.043	0.0	284.768	u.0	29.003
49.200	\$1600.000	910.514	1.201	0.242	٠.٠	223.244	-50.078	25.000
49.200	31600.000	53/-143	164	0.265	ن د د	473.450	-30.482	25.000
45.200	48106.600	515.435	1.502	4.549	9.0	220.468	-30.1.00	25.000
				*****	~.•	*******	701100	2 75 000
45.300	10343.003	524-511	٥. ٥	0.002				
					0.0	144.710	u _ 0	1.000
45.300	11 cas. cw	53/4/32	9.114	C- 414	4.0	224.457	-30.647	:
45.300	33600.000	535.424	J.642	1.2.1	0.0	220.065	- 30 . 0 94	1.000
45.300	48700.000	342.644	4.205	4. CU4	4.0	220.715	-30.104	4.400
						• • • • • • • • • • • • • • • • • • • •		
443.140	10440.000	9 30 34	J. 0	0.423	U.0	204.725	0	*** ***
441.140	31000.000							239.000
		\$ 13. 360	4.423	1.120	ç. o	945.147	-36-422	239.0W
403-143	11000000	914.016	3.82	1.362	0.0	241-211	- 344.653	235.000
443.140	48166.603	500.502	4.855	4.473	4.0	441.533	-430.008	215.000
471.250	10540.003	5 10. 434	J. 0	3. 149	U.C	173.400	u. 0	555.030
4/1.250	Micc. Cod	5110/61	0.144	0.409	0.3	442.404	-204-176	535.440
473.230	33800.000	940.042						
			3.1/1	0.356	0.0	455.477	-3-1-4-4	\$\$5.000
413.253	48100.003	344.044	4.422	0.124	u. 0	334.439	-300.471	555-000
441.293	10443.333	510.462	J. 0	-4.043	0.0	131.904	J.0	210.000
401.270	31 (513.119	1,251	-1.150	0.0	166.110	-39.150	230.000
403.240	33663.063	530.638	0.543	-1.363		127.796	-45-828	
483.240	48700.003	542.435			0.6			230.000
447,240	48 100 6 5 3 5	344.437	3. 174	-2.329	9.0	413.470	-201-305	230.000
48.100	10940.600	510.012	0.0	O. C10	0.0	131.949	U. U	5.000
48.143	31000000	310.6/4	3.401	-1.440	0.0	141.007	-4-418	3.000
46.133	31000.000	510.700	0.005	-1.645	0.0	141.870	-4.473	5.000
48.100	48160.600	535.415	-4. 789	-4.459				
			-40107	-9.4.74	0.0	141-057	-4-644	5.400

48.200	31 COO. COO	937.031	4.108	0.352	0.6	442.501	-10-604	25.000
46.200	33666.000	537.175	3.147	0.416	0.0	145.744	-13.772	25.034
48.200	46100.600	510.444	-0.334	0. 648	0.0	141.894	-4. 922	25.000
44.200	44100000	72044VV	4.351	•••••	•••	• • • • • • • • • • • • • • • • • • • •		
	10543.600	530.925	0.0	0.02	0.4	131.972	V.0	1.000
48.250				2.670	0.0	194.644	+62.117	1.033
48.250	31 600-003	339-132	4.176		6.0	141.121	-63. 149	1.033
44.250	3300.000	543.731	1.036	3. 553			~391.240	1.000
44.250	44166.663	544.474	4.313	12.201	0.4	479.251	-344.280	
48.300	10540.000	532.313	0.0	1.088	0.0	134.944	u.0	* 20 . JUG
48.300	31 (60.663	541.335	9.922	1.434	0.6	3/8.463	-243.535	620.010
48.330	33160.000	542.411	1.142	1.744	0.4	415.094	-244.146	420-003
48.100	48100.000	541.0/8	1.201	0.434	0.6	441.194	-850.450	624.033
							u.0	10.000
493.420	10540.003	512.467	5.6	0, 454	0.0	142, 194		10.000
491.423	31000.000	542.112	4.245	0.511	0.6	237.627	-13>.429	
443.420	31663.600	243.205	1.101	J. 614	0.6	3 04 . 3 36	-424-134	10.633
493.420	45746.046	5>3.161	1.068	0.443	٥.٥	476.192	-244.555	10.000
53.352	10143.003	530.134	٥.٥	3.448	0.0	257.500	٥ سو	540.000
50.352	310,3.0.3	243.218	1.503	1.744	i.i	320.070	-64.115	340.600
			1	1.410	0.6	351.484	-42.446	\$40.000
51.352	3160	344.43/		0.154	0-4	4/0,418	-214.374	540.000
\$1.352	40300.000	551.115	6.206	0.174	0.4	470,430		
\$13.450	10:40.000	557.716	3.0	0.664	0.0	215.304	. .0	640.000
513.650	31,01.003	353.433	7.415	0.535	0.0	214.000	-30.100	643.003
	336	3434/43	3.998	0.473	J.0	247.176	-51.442	
313.653	45704.643	551.243	5.60	0.444	6.6	334.234	-54.725	404.020
\$13.470	43772.000	,,,,,,,,,	,,,,,,	*****	•••			
323.600	10542.003	5 10 . 276	J. 0	1.061	0.0	119.435	u. 0	\$40.0 ~
\$21.800	311.2.1.6.0	5+6.224	1.446	1.353	U. G	471.147	-151.102	146.223
521.800	\$1003.643	541.101	J. 777	1.343	0.0	4/5.283	-155.474	143.000
\$23.000	48736.663	332.140	3.344	1.143	6.0	515.711	-250.312	140.000
\$33.920	10940.000	310.673	3.0	0.344	0.6	174.854	u. 0	640.600
531.423	31666.603	540.134	1.414	-0-123	0.6	47 881	-20>.423	**7.00
\$33.920	3360600	347.444	4.545	-0.114	J-0	514.934	-120.410	443
\$33.923	48763.603	552.414	3.744	4, 657	U - D	634.772	-46 us = L4	*******
	10540.022	344.414	3.0	0.839	3.6	147.557	v.0	\$43.000
33.130				1.320	0.0	444.461	-13/-008	543.000
53.400	31 (60.00	547.423	1.904		0.0	4 47 . 903	-15/.424	344.000
53.100	110.0.0.4	546.265	3.844	1.231			- 13/. 693	540.000
33.103	44 100-600	553.346	5.141	U- 5 82	0.6	530.052	- 131.007	340.000
11.20	14540.003	534.454	د . د	0.604	0.0	142,457	۵ 👡	5.000
33.200	31	541.447	7.649	-3.316	U - C	444.400	-131.401	9.000
51.232	33602.003	144.102	1.075	-3.105	U. C	444, 483	-351.024	3.000
\$3.200	487.3.003	551.265	5.444	0.269	4.0	504.075	-35/.044	5.000
						142.462	۵. د	19.000
53.330	10940.000	534. +64	3.4	3.611	6.6		- 157.004	13.630
33.300	11000.000	541.192	1.943	0.215	0.6	479.966	-354.025	15.000
58.330	31660.000	544.374	2.472	0.172	J.4	499.401		
33. 100	4 # 1 Cu. Cu3	553.712	5, 354	0.046	0.0	500.455	-351.644	15.000
544.023	10443.003	554.4/3	٥.٥	0.601	G. G	142.942	٠.٥	1.000
\$44.623	11.003.003	5-1./	4.294	0.302	0.0	4 19 . 9 7 9	-351.012	1.030
		341.446	3.122	0-132	J. 6	444.449	-378.024	1.300
544.0.3	116.0.000	553.536	5.050	-0.176	0.0	50033	-35/2442	1
\$44.023	48160.003	273.750	,	*****				
\$4.103	10:40.003	514.108	٥.٥	-0.CM2	0.0	140.344		100.000
\$4.100	316-3.633	547.105	1.716	-4.463	0.0	311.384	-111.059	100.000
\$4.100	316.0.0.00	540.317	J. 65 j	-6.464	0.0	321.574	-145-213	100.000
54.100	4.00.000	552.440	4.143	-4.454	4.0	341.425	-391-245	100.000
-								\$.000
54.200	10540.000	539.313	J. J	0.005	0,0	140.142	٠.٥	7.000

340 200	33800.000	240./60	1.004	~[.237	0-0	301.900	-164.616	5-300
54.200	487GJ.LOJ	552.411	4.216	0.097	U.C	341.955	-204.588	5.000
						2		,,,,,
54.300	10540.00	\$ 19.428	0.0	0.034	0.0	143 380		
\$4.300	31600.600	501./53				143.340	u_ 0	25.000
			4.331	2.632	0.6	323.54#	-189.208	25.000
54.300	33600.633	204.310	0.544	1.544	U- G	330.415	-144.422	25.630
54. 300	48166.603	553-214	4.514	0.241	0.5	341.958	-201.547	25-000
554.053	14592.603	534-432	4_0	0.654	6.6	140.191	u_ 0	1.000
554.053	310.0.000	544.714	287	0.563	0.0	314.535	-140.146	1.000
554.050	336.6.667	5.4.1.4	3.416	0.640	0.0	341.523	-201.532	1.000
554.054	48 740.000	551-152	3. 154	-4-643	9.6	391.957	-201.500	
		******	*****	- 04 447	440	341. 177	-2019 300	¥-000
\$64.090	10540.600	5 14.444					_	
364.013			0.6	0.435	0.0	204.543	u. 0	211.000
	31 600.666	941.853	4.543	1.141	0.0	308.705	-189.120	211.000
564.040	33620.603	>>>>	J.5.5	1-251	0.6	369.357	-100-770	211.000
544.643	467.1.000	554.566	4.143	1.434	0.0	344.645	-160.462	211.400
514.140	10440.653	543.350	2.0	0.492	0. C	323.701	u. J	525. 300
5/4-173	\$1000.C.M	553-271	1.412	0.411	6.6	334.510	-14.879	929.000
\$14.143	33603	554.672	0.0.2	0.4.1	6.0	111.5.0	-14.00>	
5/4.190	40/00.003	534.V/6	4.104	0.300				525.000
,	********	,,,,,,,	4.104		0.0	342.544	-14.647	929.010
944.210	1041				_			
	10.40.669	542.855	0.0	0.421	6.c	3/1.122	J. 0	420.000
584/3	91 Cu2. Cu3	150.122	7.631	J. 452	a.c	354.138	-10.560	424.000
384.275	3346	551.336	3.014	J. 484	u.c	377.443	-10.742	420,000
300/4	44100.000	353.334	4.116	0.2/4		404.131	-34.939	420.000
394.340	13940.013	591.113	0.C	0.245	0.0	801.200	u. 0	410.433
\$ ** . 340	11001.003	551.1.6	13.310	2.424	0.0	1072.144	-661.513	
300.300	1160	334.210	J-473	0.400				410.000
544.343	441636663	>>0.107			٥.٥	1504-412	-444.298	444444
,,,,,,	441632003	>>0.101	4.314	0.412	0.6	1004.001	-11-142	4/4,300
4.548	10.40.003	443.611	J.J	-0.313	0.0	104.395	u. 0	1020.000
4.542	17406.633	554.244	20.41.	0.649	0.0	1511-772	-1402.347	2 - 25 30
4.544	14461.613	511.411	3.413	0.645	٧.٤	4514.417	-1410-524	1070.00
4.548	36100.600	550.245	4.134	0. 614	U.0	1752.447	-1000.222	1000.000
				-		*		
4.544	10941.603	543,478	0.0	-0.313	u.c	131.001	u_0	1.000
4.559	114.,	551.297	40.171	U. (U S	0.0			
4.119	146.0.00	551.313				1511.772	-1410.132	1-000
4.54			0.669	3.60	0.0	1214.411	-14120191	1-000
4.744	34103.603	\$50.244	4.130	0.663	0. C	1154.641	-1050.931	1.000
414.550	13443.003	5 44 . 6 4 4	J. J	0.146	0.0	102.916	ل مد	20.000
414.550	1/202.603	ううしょごうし	17.471	43. Cu?	J.4	1311.744	-1400.003	يدن.ن 2
614.513	14000.130	554.923	J. 647	0.002	J.0	1514.945	-1414.009	20.411
444.550	34165,000	556.253	4.330	J. CU1	v.a	1752.612	-4667.716	20.000
						•••	,	
4.551	10940.000	541.176	٠.٠	0.532	0.0	107.533		
4.571	1/000.000	254.442						4.000
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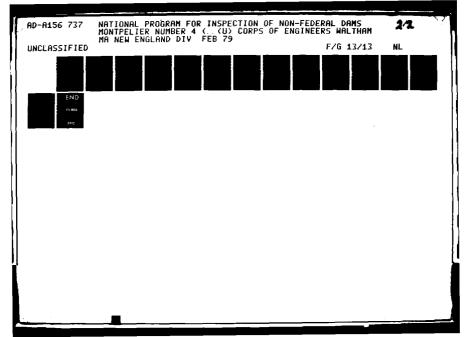
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APPENDIX E

Information as contained in the National Inventory of Dams





MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS 1963 A

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语言语 INVENTORY OF DAMS IN THE UNITED STATES

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This Phase I Inspection Report on Montpelier Number Four has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

OOSYPH W. FINEGAN, JR., MEMBER
Water Control Branch
Engineering Division

Joseph J. Mc Elroy

JOSEPH A. MCELROY, MEMBER
Foundation & Materials Branch
Engineering Division

Carney H. Vazian

CARNEY M. TERZIAN, CHAIRMAN Chief, Structural Section Design Branch Engineering Division

APPROVAL RECOMMENDED:

Jan B. Fryar DE B. FRYAR

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Joseph a. Mc Elroy

Water Control Branch Engineering Division

JOSEPH A. MCELROY, MEMBER
Foundation & Materials Branch
Engineering Division

CARNEY M. TERZIAN, CHAIRMAN

Comey 4. Vazian

Chief, Structural Section

Design Branch

Engineering Division

APPROVAL RECOMMENDED:

OE B. FRYAR



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02164

REPLY TO ATTENTION OF: CHOED-E

July 13 15 -

Green Mountain Power Corporation Contpelier Vennon: 05802

Genelemen:

Varwaries herewith for your information and use is a copy of the Inspection Report on the Montpelier No. 4 Dam. This inspection was made under the authority of Public Law 92-367 by the firm of Dufresne-Henry Engineering Corporation, North Springfield, Vermont under the direction and supervision of the Corps of Engineers. A copy of the finished report has been forarded to the Governor and the Department of Water Resources, the cooperating agency for the State of Vermont.

Section 7 of the report contains an evaluation and recommendations. If you have any questions concerning this report, contact the Department of Water Resources first. Then, if there are further questions of that the Project Management Branch, Engineering Division of this office. We thank you for your cooperation and assistance in carrying out this program.

Sincerely yours,

Incl As Stated JOE B. FRYAR



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM MASSACHUSETTS 02164

REPLY TO ATTENTION OF: NEOTON ATTENTION OF:

33 13 373

Green Mountain Power Corporation Montpelier Vernoni 95872

Gentlamen:

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Sincerely yours,

Incl As Stated JCE B. FRYAR



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02154

PEPLY TO STITES ON OFF MEDICIDEE

Mr. Casinald A. LaRosa, Acting Commissioner Copariment of Meter Resources State of Mermont Montgelier, Versont 05602

Dear Mr. LaRosa:

Forwarded herewith for your information and use is a copy of the Phase I Inspection Report on Montpelier No. 4 Dam. This inspection was performed in accordance with Public Law 92-367 under the direction of the Corps of Engineers. Copies of the finished report have been forwarded to the Governor and the owner. We thank you for your cooperation and assistance in carrying out this program and hope this report will help you to develop an effective dam safety program.

Sincerely yours,

Incl As stated 📶 DE B. FRYAR



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD

WALTHAM MASSACHUSETTS 02154

REPLY TO ATTENT IN OFF NEDERAL

Mr. Reseast to Mesons, Acting Commissioner Department of Meson Pesources State of Mesont 105002

the Mr. Lakosa:

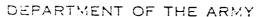
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Sincerely yours,

🥕 105 8. FREAK

Chief, Engineering Division

local As wasted



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD

WALTHAM, MASSACHUSETTS 02154

charable Richard A. Smelling Gunstmor of the Pasta of Marmont State Caritol Compeliar, Merhont 05602

Tear Severaer Smelline:

I am forwariing to you a copy of the Montpelier No. 4 Dam Phase I Impedition Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is invited to the biginning of the report. I have approval the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow—up action is a vitally important part of this program.

Copy of this report has been forwarded to the Department of Water Chourses, the objecting openon for the State of Mannest. In Ifilition, a copy of the report has also been formished the owner, Green Mountain Power Corporation, Montpelies, Vermont 03:01.

Capies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this latter.

I wish to take this opportunity to thank you and the Department of Water Resources for your cooperation in carrying out this program.

Sincerely yours,

Statetin Indepe



NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02154

304 1 5 1919

Honorable Richari A. Shelling Governor of the State of Vermont State Capitol Montpelier, Vermont 05602

Dear Governor Smelling:

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Sincerely yours,

Inol Sa stated

Colonel. Coers of Engireers

Division Engineer

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